



BEHAVIOR OF REINFORCED CONCRETE COLUMNS CONFINED WITH PRESTRAINED SHAPE MEMORY ALLOY PLATES

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ABSTRACT:

External confinement of concrete columns increases the column capacities. This is essential for deficient columns to restore their named capacity, or for safe columns to enhance their capacities if required. This paper introduces a simple technique using the superior properties of shape memory alloys (SMA) to actively confine circular concrete columns. The SMA plates were tensioned prior to confinement and placed in their exact locations. The temperature change was applied after the plates were placed on the columns, leading the SMA plates to compress the columns.

The experimental program consisted of six circular concrete columns specimens (100 mm diameter, 600mm height). The parameters studied in the experimental program are the applied tension force to the external stirrups prior to the confinement process, and type of confinement (active or passive). The increase of the column capacities due the active SMA confinement was observed, as well as the change of the column behavior after active confinement. Finite element models were performed using ABAQUS software to verify the experimental results. The paper concluded an increase of (17%-38%) in the column capacities compared with the unconfined column. The finite element analysis showed close results to the experiment program, which could lead to a future wide range study to generalize this simple technique on different concrete columns.

KEYWORDS: Active Confinement, New Technique, Smart Materials, Circular Columns, External Stirrups, Prestressing, Reinforced Concrete.

سلوك الأعمدة الخرسانية المسلحة المحاطة بألواح السبائك الذكية سابقة الإنفعال

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الملخص:

التدعيم الخارجي للأعمدة الخرسانية يزيد من قدرة الأعمدة على تحمل الإجهادات. و يكون التدعيم ضروريا في حالة الأعمدة التي بها عيوب لاستعادة كامل كفاءتها مرة أخرى، و كذلك للأعمدة الآمنة لرفع كفاءتها إذا كان ذلك مطلوبا. تقدم هذه الدراسة أسلوبا بسيطا لاستغلال الخواص المتفردة للألواح السبائك الذكية لإحاطة الأعمدة الخرسانية إيجابيا، حيث تم شد ألواح السبائك الذكية بنسبة الإنفعال المطلوبة قبل وضعها على الأعمدة، ثم تمت إحاطة الأعمدة بالألواح في الأماكن والمسافات المطلوبة، ثم تم تسخين الألواح لتضغط على الأعمدة الخرسانية و هي تحاول الرجوع لطولها الأصلي. تم عمل برنامج عملي مكون من ستة عينات أعمدة خرسانية دائرية بقطر 100 مم و طول 600 مم، و تم تغيير قوة الشد للألواح و كذلك نوع الإحاطة (سلبيا و إيجابيا). تم مراقبة نسبة الزيادة في الحمل الأقصى للأعمدة و كذلك تغيير سلوك الأعمدة مع تغيير نوع الإحاطة و قوة الشد السابقة. تم عمل نماذج رقمية للأعمدة باستخدام برنامج أباكوس ABAQUS للتأكيد على

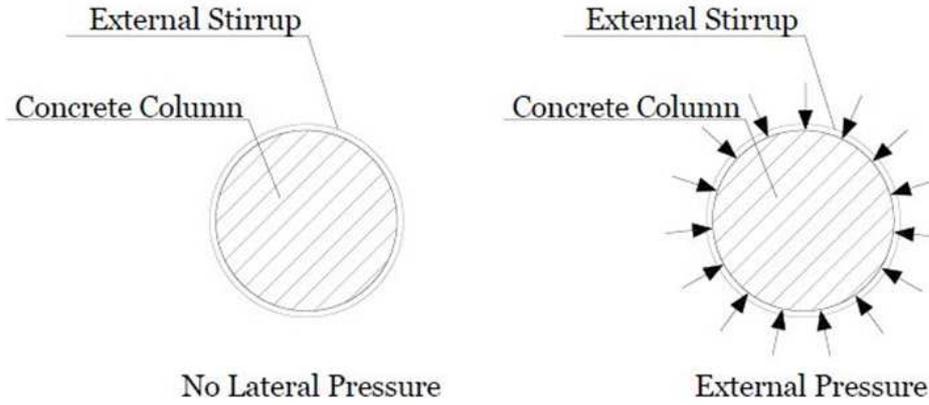
نتائج البرنامج العملي. توصلت الدراسة إلى التأكيد على فاعلية المواد الذكية في الإحاطة الإيجابية و السلبية للأعمدة الخرسانية و تم رفع كفاءة الأعمدة بنسب تتراوح من (١٧%) و حتى (٣٨%) مقارنة بالعمود غير المحاط. و أظهرت النماذج الرياضية توافقا جيدا مع النتائج العملية مما يمهد الطريق لعمل دراسة نظرية موسعة باستخدام النماذج الرياضية لتطبيق طريقة الإحاطة المقترحة على أعمدة خرسانية مختلفة.

الكلمات المفتاحية: الإحاطة الإيجابية، طريقة جديدة، المواد الذكية، أعمدة دائرية، كانات خارجية، سبق إجهاد، خرسانة مسلحة.

1. INTRODUCTION

Enhancement of column capacities could be essential if columns are proven to be deficient due to design or construction mistakes. Moreover, some loads on columns could increase due to change of building use or increase of number of floors.

Column confinement is an effective method to increase the column capacities. The confinement techniques are divided into two main types: passive and active confinement. The main difference between the two methods is the application of external pressure in the active confinement method, while passive confinement depends only of the side elongation of concrete against the external stirrups as shown in **Fig. 1a. and 1b.** (Shin, M. and Andrawes, B. (2010), Andrawes, B. et.al. (2010)). Although active confinement gives better results, the simplicity of the passive technique makes it preferable in the strengthening works.



a) Passive Confinement using External Stirrups b) Active Confinement using External Stirrups

Fig. 1: Schematic External Confinement of Concrete Columns

Passive confinement has also the advantage of having settled and verified equations to predict the percentage of capacity increase. Several models have predicted this increase. These models expressed the capacity increase in the form of increased concrete compressive strength (f'_{cc}). The new compressive strength depended mainly on the original compressive strength of concrete, the spacing, yield strength and number of external stirrups. The following relation for strength of prediction of strength increase in passive confined concrete was predicted by Mander et al. (1988) and is used in the American Code of Practice ACI 440.2R-17 (2017):

$$f'_{cc} = f'_{co} \left(-1.254 + 2.254 * \sqrt{1 + \frac{7.94 * f'_l}{f'_{co}}} - 2 \frac{f'_l}{f'_{co}} \right)$$

$$f'_l = \frac{1}{2} k_e \rho_s f_{yh}$$

$$k_e = \frac{\left(1 - \frac{s'}{2d_s}\right)^2}{1 - \rho_{cc}}$$

$$\rho_s = \frac{A_{sp} * \pi * d_s}{\frac{\pi}{4} * d_s^2 * s}$$

where;

f_{cc} = the strength of confined concrete at failure, f_{co} = unconfined concrete compressive strength, f_l = effective lateral confining pressure, f_{yh} = the yield strength of transverse reinforcement, k_e = the confinement effective coefficient, ρ_{cc} = the ratio of the area of the axial steel to the area of the core of the section, s' = the clear spacing between the spiral, ρ_s = the transverse reinforcement ratio, A_{sp} = area of transverse reinforcement bar, d_s = the diameter of the spiral between bar centers, s = the spacing between the stirrups.

This paper introduces an active confinement technique, which is proven effective, yet easy to gain the advantages of the active method in a simple way. This is achieved through the use of external stirrups made out of shape memory alloys. Prestrained SMA-plates are put on the concrete columns and heated to return to their original shape. Through this process the required external pressure is applied on the columns and active confinement is reached.

A previous research program (Atia, E., (2019)) compared using SMA-plates in the active confinement of columns with other methods, but used a different technique by applying the pressure through bolt stressing. This ignored the special properties of SMA, which are studied here.

2. THEORETICAL BACKGROUND

In this section the general concept of SMA is briefly explained. The special type of SMA-plates used in the experimental program is also introduced.

2.1. Properties of SMAs

SMAs are able to undergo large deformations (up to 8%) and return to their original shape upon heating. These special properties are mainly due to the back and forth transformation between the martensite phases and the austenite phases on the atomic level. The austenite and martensite phases are closely related to the state of the SMA in its high temperature and low temperature. The SMA's are dependent on two transformation temperatures, austenite finish temperature (Af) and martensite finish temperature (Mf) (Abdelrahman, K. and El-Hacha, R. (2015)). **Fig. 2** shows the changes of strains of the SMA related to temperature change.

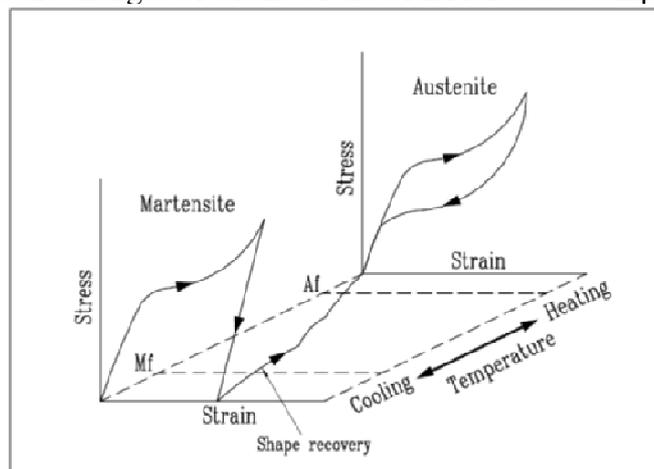


Fig. 2: Mechanical Behavior of SMA

At temperatures below M_f where the alloy is in its martensite phases, the SMA behaves plastically. If the SMA is heated above the temperature A_f , the alloy transforms to austenite and the SMA recovers its original shape (super-elasticity). That phenomenon is known as shape memory effect (SME) (Shin, M. (2012)). The SME phenomenon is associated with large stress recovery at restraint of the SMA to restore its original shape. This level of stress is closely related to the material composition, and the amount of prestrain. Due to this reversible process, SMAs maybe used in several applications and appears to be one of the main materials of the future. SMAs have already been used for multiple applications within the biomedical field (eyeglasses, dental wires, and reinforcement for arteries and veins), and aerospace industries (fixed wing and rotary-wing components) in the past. Recently, they

have also started to attract a lot of attention in other engineering fields due to the price decrease that has occurred over the past decade. This price reduction is expected to continue within the near future and should radically continue due to the eventual increase in both iron and copper-based alloys (Park, J. et.al. (2011), Choi, E. et.al. (2013)).

2.2. SMA plates used in the Experimental Program

SMA have different types depending on the chemical composition of the alloy. The used alloy consisted of Nickel (Ni) and Titanium (Ti) with an atomic percentage of 55% and 44%. The SMA plates could provide a linear strain recovery up to 8% when heated above a temperature of 23°C. This temperature range was specifically chosen because it is easy to reach. The SMA plates have been supplied by the manufacturer in its martensitic pre-strained stage. According to the manufacturer, the ultimate tensile strength, elastic modulus, and strain at failure of the SMA plate were 896MPa, 84821MPa, and 18%, respectively.

3. EXPERIMENTAL PROGRAM

Six RC columns with overall dimensions of 100 mm diameter, 600mm height were tested. The molds were precisely cut from a polyvinyl chloride (PVC) plastic pipe, such that their inside surface was smooth and they were all very similar in geometry. A concrete mix targeting 22.5MPa at 28 days was used in casting the columns. The longitudinal and transverse reinforcement of all specimen was 4 #10mm bars longitudinally and #4mm diameter bars at 85mm spacing in the transverse direction. Three additional #4mm stirrups and one layer of CFRP at 100mm width were applied at the beginning and end of each column to avoid local failure. The external stirrups were SMA-plates with the dimensions and shape shown in **Fig. 3**.

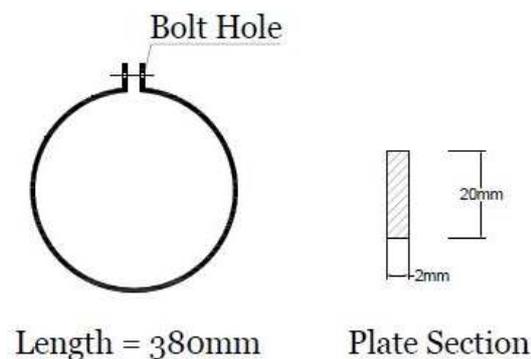


Fig. 3: Dimensions and Shape of SMA- Plate

The test specimens were divided into five columns and a reference column depending on the level of pretension force of the external stirrups as shown in **Table 1**, while the reinforcement details are shown in **Fig. 4**.

Column (C-1) was considered as a reference specimen, without any type of confinement. Column (C-2) was passively confined and the temperature of external stirrups was smaller than 15°C. Column (C-3) was passively confined and the temperature of external stirrups was bigger than 23°C. Columns (C-4, C-5 and C-6) were actively confined, the temperature of external stirrups was higher than 23°C and tension of external stirrups by 2000, 4000 and 6000 N respectively.

The tests were performed in the RC Lab., the Faculty of Engineering, Al-Azhar University. The hydraulic jack used in testing was with capacity 100 ton. **Fig. 5** shows the general setup of the test frame, the hydraulic jack and the specimen.

Table 1: Specific Parameters for Column Specimen

Specimen #	Spacing of External Stirrups (mm)	Temperature ($^{\circ}$)	Type of Confinement	Pretension force (N)
C-1		Reference Column		
C-2	85	≤ 15	Passive	0
C-3	85	≥ 23	Passive	0
C-4	85	≥ 23	Active	2000
C-5	85	≥ 23	Active	4000
C-6	85	≥ 23	Active	6000

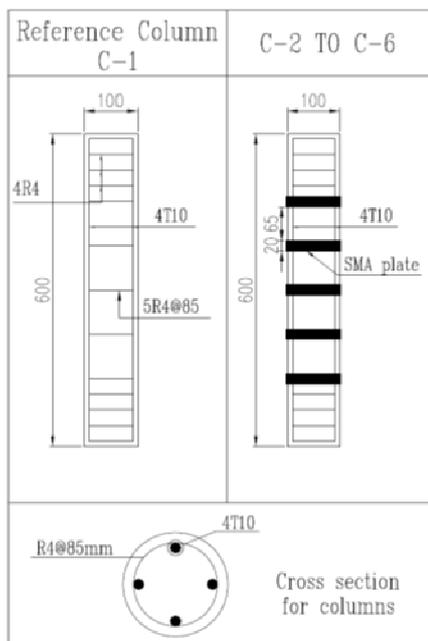

Fig. 4: Details of Reinforcement for All Columns

Fig. 5: General Test Setup

The SMA-plates were mounted on a fixed rig and tensioned using a torque wrench to the required stress. This procedure was controlled by gluing a strain gauge on the tensioned plate. After reaching the desired strain the plates were placed on the columns and attached by a bolt, and heated by pouring ordinary hot water to achieve the required pressure on the concrete column. The tensioning procedure is shown in **Fig. 6**.

4. TEST RESULTS

Columns were tested under static load and the results, cracks and behavior were observed and recorded precisely. Testing showed an increase in the column capacity of about 17% for the passively confined column (C-2) up to 38% for the specimen (C-6). The external stirrups served in arresting the failure cracks. The actively confined columns showed more ductile behavior and less cracks before failure. **Fig. 7** shows crack pattern of all columns.

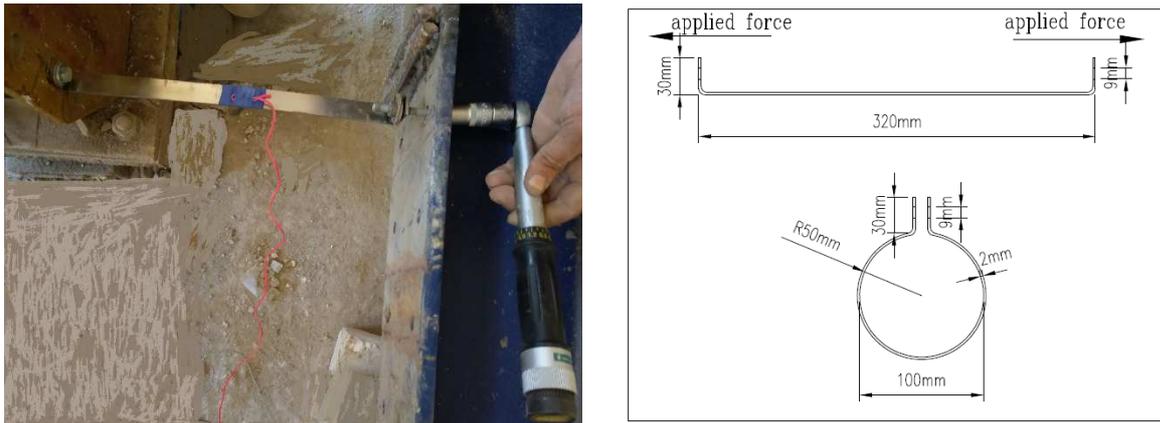


Fig. 6: Tensioning Procedure

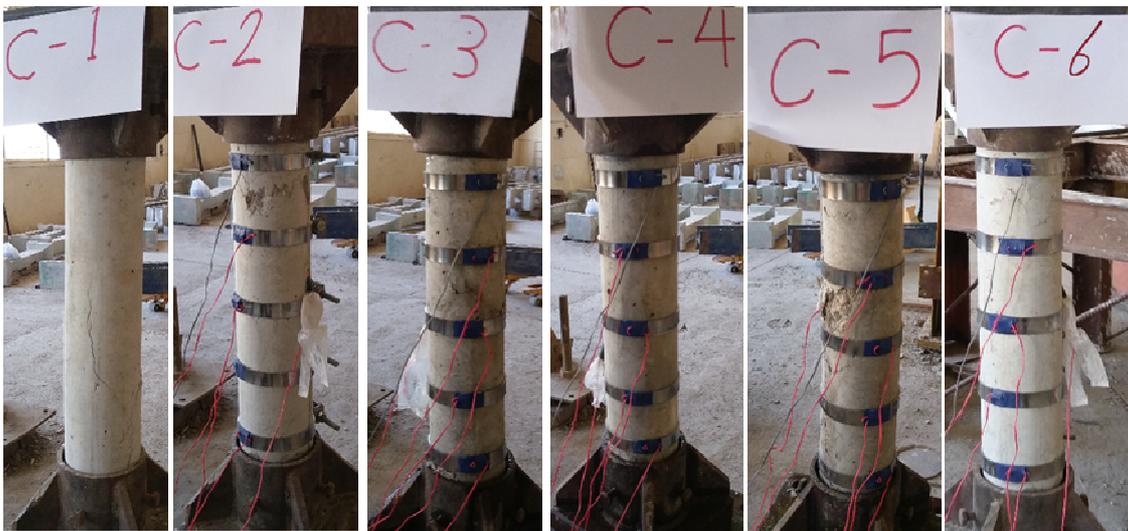


Fig. 7: Crack Pattern for All Specimen

4.1. Steel Strains

The longitudinal steel strains were obtained from the electrical strain gauges. Fig. 8 shows the load steel strain curves, through the load history, for the all columns at the first third span section. The actively confined columns reduced the steel strains in the longitudinal column reinforcement at the same load level. The higher the preloading level of the SMA-plates, the lower the steel strains at the same load level.

4.2. SMA-Plates Strains

The SMA strains at the mid height of the columns were obtained from the electrical strain gauges. Fig. 9 shows the load SMA strain curves through the load history. The pretensioned SMA-plates showed similar behavior regardless the initial prestrain of the plates, with low strains even at high load levels. While the non-preloaded SMA-plates showed quicker response and larger strains at the same load levels.

5. ANALYTICAL MODELS

The specimens were modeled using finite element analysis on the well-known software ABAQUS. The analysis was based on the non-linear iterative secant stiffness formulation. Concrete Damaged Plasticity model was used to describe the yield criterion of concrete as compressive behavior and tension behavior .

The reinforced concrete column was idealized using three different types of elements. Concrete was represented by an eight-node solid element (C3D8R) with 24 degrees of freedom. The reinforcement was represented by a truss element (T3D2) with 6 degrees of freedom. SMA was represented by an eight-node solid element (C3D8R) with 24 degrees of freedom. The contact between concrete and SMA was represented by tie behavior.

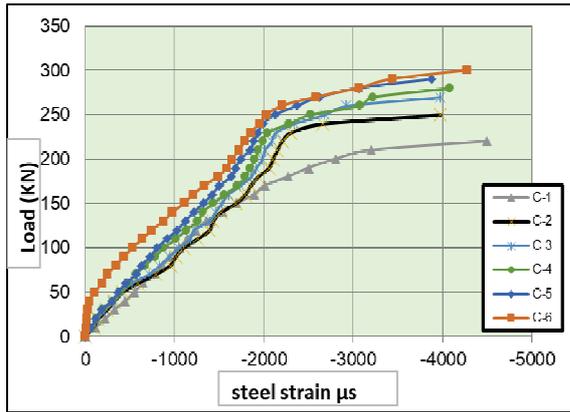


Fig. 8: Comparison between longitudinal Steel strain at first third for all columns

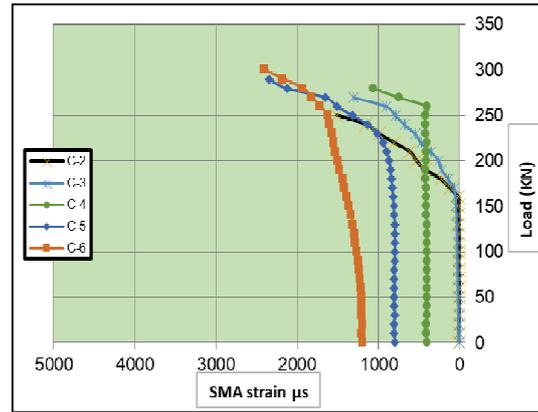


Fig. 9: Comparison between longitudinal SMA strain at midpoint for all columns

Fig. 10 shows the deformed shape of the model of the control column (C-1). The verification of the numerical model has shown good agreement with the experimental results. Comparison between experimental and analytical results is shown in Table 2.

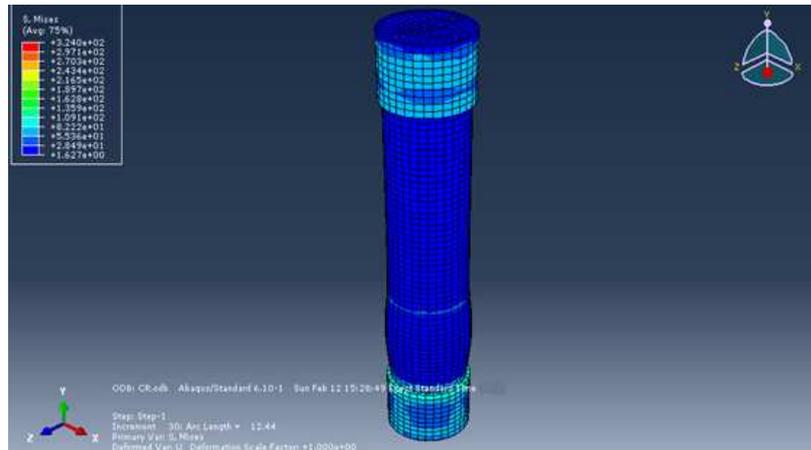


Fig. 10: Deformed Shape of Analytical Model of Specimen (C-1)

Table 2. Comparison between the Experimental and FEA Failure Loads

Specimen #	Failure Load (kN)		Precision
	EXP.	FEA	
C-1	220	226	1.03
C-2	258	261	1.01
C-3	277	278	1.00
C-4	280	285	1.02
C-5	290	292	1.00
C-6	302	299	0.99

5.1. Steel Strains

The steel strains were obtained out of ABAQUS for the different specimens and compared with the steel strains in the experimental program. The graphs showed good agreement especially in the plastic stage of the load history. Fig. 11 shows the load- steel strain curves for each specimen, obtained from FEA and experimentally.

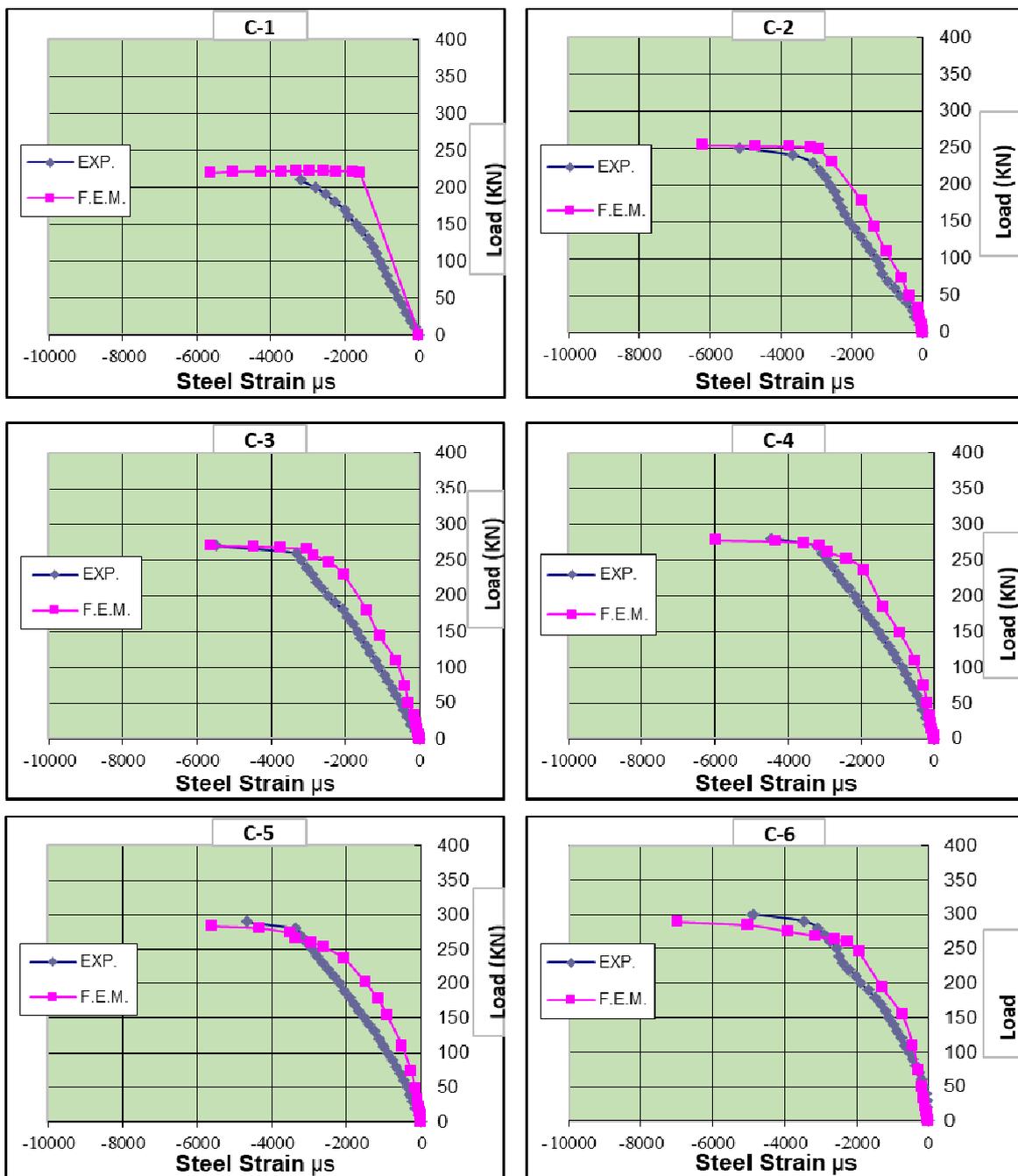


Fig. 11: Comparison between Experimental and Analytical Steel Strain for All Columns

5.2. SMA-Plate Strains

The SMA-plate strains were obtained out of ABAQUS for the different specimens and compared with the SMA-plate strains in the experimental program. The graphs also show very good agreement all through the load history of the specimens. Fig. 12 shows the load-SMA strain curves for each specimen, obtained from FEA and experimentally.

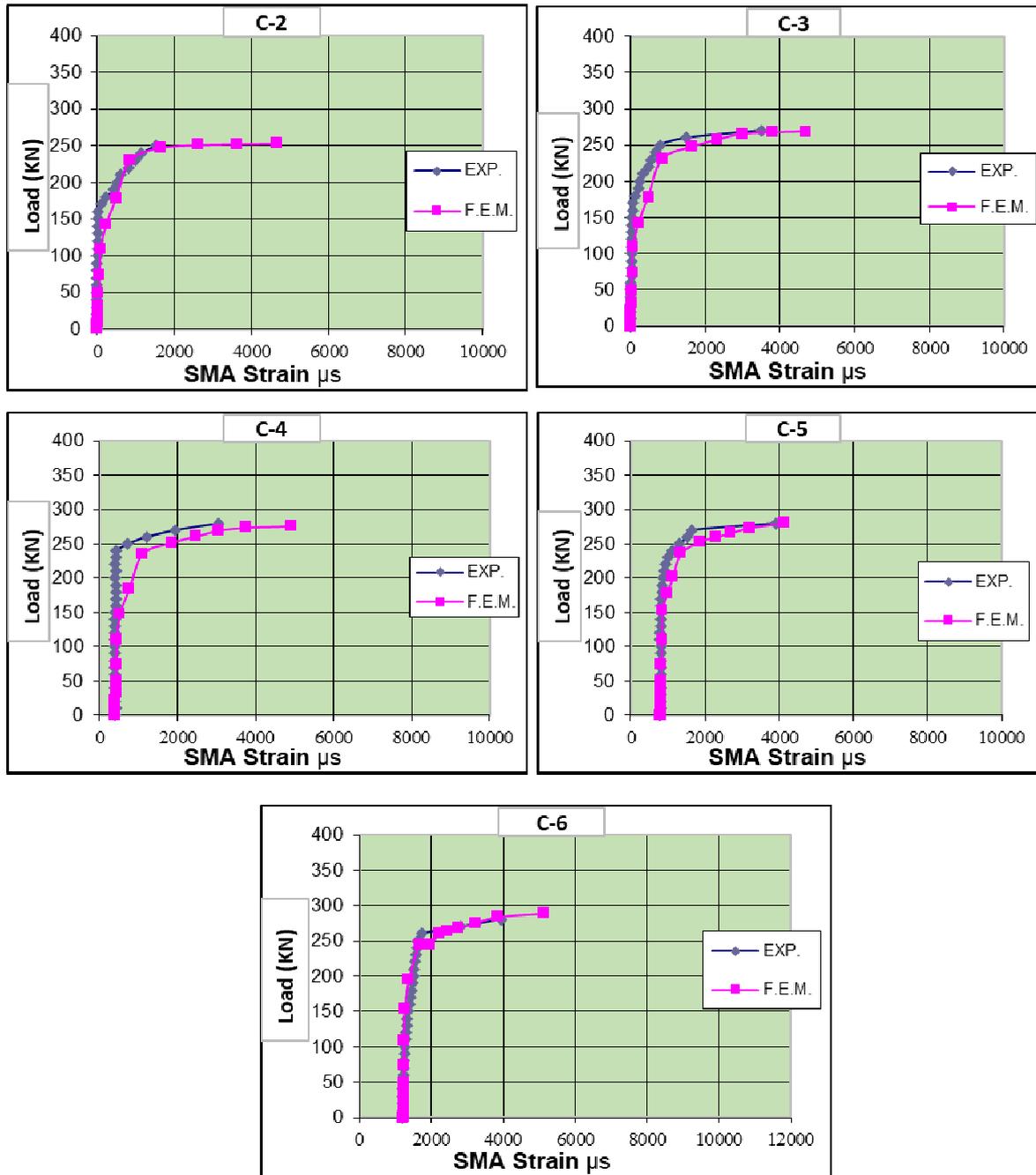


Fig. 12: Comparison between Experimental and Analytical SMA-Plates Strain for All Columns

SUMMARY AND CONCLUSIONS

The present study investigated the effect of active and passive confined concrete columns using a simple technique to apply active confinement with SMA-plates. The following summarizes the findings of this investigation:

1. SMA-plates were proven to be an efficient and simple way to actively confine concrete columns.
2. The increase of the column capacity started with 17% using passive confinement, and reached up to 38% for columns active ly confined with SMA-plates.
3. Increasing tension force in the external stirrups caused an increase of the capacity of columns confined with SMA.
4. Finite element models showed good agreement with the experimental results in the capacities and strain results.

5. Finite element models also present a solid base for a future wide range study to verify the effect of active confinement using SMA-plates on different columns and obtain a methodology to calculate the capacity increase.

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