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A MICROSIMULATION APPROACH TO EVALUATE OPERAT OF WEAVING SECTIONS AT URBAN UNCONVENTIONAI INTERSECTIONS IN CAIRO

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الأونة الأخيرة ظهور طرق غير تقليدية لمعالجة بعض التقاطعات رباعية الارجل بمدينة القاهرة. طرق هذه <u>لرق</u> هذه لا تتضمن <u>أي استخدام إ</u>شارات مرورية للتحكم في التقاطع ولكنها تتضمن اغلاق الجزيرة الوسطي منع المرور المباشر من خلالها والاعتماد الكلي على فنحات الدوران الي الخلف وبالتالي تتقاطع مسارات انوي مع مسارات الطريق الرئيسي وتتكون قطاعات تقاطع المسارات على جانبي الجزيرة الوسطي. هذه إول النمذجة والمحاكاة المرورية لقطاعات تقاطع المسارات الناتجة عن هذا النوع من التقاطعات الرباعية وذلك ينامج فيزيم (VISSIM) للمحاكاة المرورية. التي اتاحت المحاكاة المرورية اختبار القطاعات تحت تأثير احجام منافة واطوال قطاعات مختلفة كما اتاحت أيضا حساب السعات السعة المرورية القطاعات تحت تأثير احجام من المعادلات للتنبؤ بالسعة المتوقعة بطريقة تحليل الانحدار. بعد حساب سعات القطاعات تمتتنافيات منافة واطوال قطاعات مختلفة حما اتاحت أيضا حساب السعات السعة المرورية القطاعات المختلفة للقطاعات من المعادلات للتنبؤ بالسعة المتوقعة بطريقة تحليل الانحدار. بعد حساب سعات القطاعات المختلفة للقطاعات من المعادلات للتنبؤ بالسعة المتوقعة بطريقة تحليل الانحدار. بعد حساب سعات القطاعات المختلفة القطاعات من المعادلات المرورية ينيونيا المورية تعليل الانحدار. معد حساب سعات القطاعات تمتتضمن الدراسة الي وذلك لمقار نتها بالقيم المحسوبة باستخدام المعادلات الموضوعة الموجودة بدليل سعة الطريق (2010) بقوذلك لمقار نتها بالقيم المحسوبة باستخدام المعادلات الموضوعة الموجودة بدليل سعة الطريق (2010) موزلك لمقار المقارات المغتلفة وحلان المعادلات الموضوعة الموجودة بدليل معا الطريق الوضي عنوا مودنية المقارنة القيم المحسوبة بالمحلكاة وجد ان المعادلات دليل سعة الطريق (2000) الموضوعة لا موزلنة معانية المقارات وذلك لأن المعادلات الموضوعة المعاد ومان الموضوعة الموضوعة الموجودة بدليل ما مرامي الموضوعة للوضوعة للعلوم في الولايات مورنية المقارنة القيم المحسوبة بالمحاكة وجد ان المعادلات دليل سعة الطريق (2000) الموضوعة للموضوعة من من المنوع من المقطاعات وذلك الموضوعة تعطي قيم مسعات مررورية غير منطقية الوضي عن من الموضوعة الموضوعة الموضوعة الموضوعة الموضوعة من من الموضوعة الموضوع ألموض ما الموضوعة عرب مناليا ما الرزي القيم من الموضوة الي المعير التغيرران الموضوة الموضو مات الموض

م استخدام المعادلات الخاصة بدليل سعة الطريق (HCM 2010) لتصميم أو تقييم اداء قطّاعات تقاطع المسارا ع في المناطق الحضرية ويفضل استخدام النماذج المذكورة في هذا البحث بعد اجراء المعايرة المطلوبة.

ABSTRACT

Unconventional <u>median U-turn intersections intersection treatments</u> have been ex implemented along major corridors in Cairo₁₇ Egypt. These <u>unconventional traintersections</u> do not involve signalization ve any signal control at any point, theypoi utilize_-a non-traversable median with a U-turn crossover at the downstream to ma crossing movements, thus movements and thus, creating a two-sided weaving between the minor approach and the U-turn crossover between the minor approach ar turn slot on both sides of the median. In. In this paper, VISSIM is-was used to m simulate these weaving sections throughout an experimental analysis with with influential factors namely; major demand, minor demand, weaving length andlength minor through traffic splitsplit (% Mi THR). The experimental design resulted scenarios whichscenario runs, which were simulated automated using through an Visual Basic program developed specifically for this studyy. Various programs w developed for the simulation output extraction and manipulation. The first stag

A MICROSIMULATION APPROACH TO EVALUATE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONV INTERSECTIONS IN CAIRO Soliman, A.K. and Abo Hashema, M.A.Soliman, A.K. and Abo Hashema, M.A.

analysis was dedicated was dedicated to estimate the capacities of to estimate the capthe weaving sections and the minor entrance whichentrance, which were found -t negatively correlation correlated. with each other. Increasing the major demand of decrease in the minor entrance capacity and an increase in the capacities of the Hestions. It -was also found that increasing capacities increase with the increase weaving length-; however, increasing the length beyond 200 meters was not be Increasing the minor through split caused an increase in volume ratio and a dec capacities Furthermore, Furthermore, regression analysis was used to develop, simulation based capacity prediction models that resulted in a relatively high R^2 val second stage of the analysis covered-was dedicated to the test the application approp of the HCM 2010 weaving methodology to when applied to the urban weaving se predict capacities, lane change rates, and speedscompute capacities, lane change weaving and non-weaving speeds for each weaving section. Comparisons bety predicted and the simulated estimates with the simulation models sshowed that 1 HCM 2010 2010 methodology provides provided higher extremely highcapacity pr up to 1.6 times the capacity estimates almost double the simulated capacitiescapaciti HCM 2010 also provided lower capacities for higher weaving lengths, which inc higher sensitivity to the increase in volume ratio and a lower sensitivity to the inc weaving length .On the other hand, Thethe developed regression models on the ot provided produced capacity more accurate estimations estimations that were more This provided evidence that the structure of the developed models were is more s represent capacities of similar weaving configurations. Further comparisons using tests and parity plots between the simulated and the predicted estimates showed that 2010the HCM 2010 methodology also underpredicted the lane change rates in the sectionrates; s and thereby therefore, producing higher speed predictions were higher simulated speeds - estimates at each weaving section. - Finally Finally, an effort undertaken carried out to calibrate and modify the speed prediction algorithms of t 2010 2010 speed prediction algorithms using the simulation data points,; however, t did not produce yield any significant results.

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Keywords: VISSIM, Microscopic traffic simulation, <u>Urban weavingUrban</u>sections, HCM

2010, Capacity analysis, Unconventional intersection treatments

INTRODUCTION

Most of the weaving related research is focused on freeway weaving section du complexity of analyzing operations with interruptions to the flow. In urban, envirous disturbances to the traffic flow could be causes by various elements such as created access, driveways, traffic signals, yield control, stop signs, pedestrians, on street parking access.

Despite, being the state of the art transportation document, the HCM 2010 also stops addressing weaving in urban areas with interrupted flow conditions. It premethodology limited to freeway conditions. Practitioners use the HCM 2010 method design and analyze alternative weaving sections. However, using it to address urban is controversial.

To date, there has not been a recognized procedure for the analysis of urban weaving however, simulation seems to be a reliable and sophisticated analysis approach extensively used nowadays to address limitation of available methodologies and complex traffic phenomenon. VISSIM traffic simulator is a very sophisticated time behavioral based simulation model developed to model urban traffic, and public tra addition to pedestrian's flows. VISSIM offers flexibility in several respects. The seprogrammability overcomes the limitation of the graphical interface. Also the concep

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Figure 2 illustrates the intersection treatment.	-1 ⊑
Flow components and the formation of weaving sections	<u>L</u>
<u>metted</u> <u>The HCM 2010 [1] defines weaving as the crossing of two or more traffic streams weaving as the crossing as the </u>	
the weaving section experience turbulence in excess of the normally present o	n
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In Cairo, many urban intersections have been treated with U-turns as shown in figure	<u>e</u>
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handle all crossing movements. This treatment does not involve any signals contraction point. Conflicts between traffic streams are managed through yield signs, therefore	<u>1(</u>
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As a consequence of implementing this type of design, direct left turning vehicle minor street are forced to make a right turn followed by a U-turn (RTUT). On the oth minor through traffic (% Mi THR) are forced to make a RTUT and then a right turn (the minor street, downstream of the U-turn crossover. Left turning traffic of the ma must also utilize the median side lanes to make a U-turn followed by a right turn (UT the minor approach. The RTUT movement requires a series of lane changes to reach most lane towards the U-turn crossover; similarly, UTRT movement must change reach the outer lane toward the downstream minor approach.

Figure 2 shows a close up of the analysed intersection, and illustrates the formatic urban weaving sections resulting from the local intersection treatment. Ramp to ran and ramp to arterial (R-A) flow components are also shown in the figure.

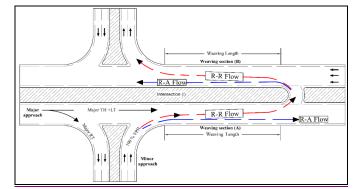


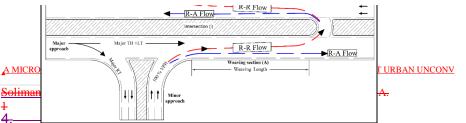
Figure 222; Formation of weaving sections at intersection (i) and flow components.

Most of the weaving related research focused on freeway weaving sections due to the com analyzing operations with interruptions to the traffic flow. In urban, environments, disturban traffic flow could be caused by various elements, such as cross street access, driveways, traffi yield control, stop signs, pedestrians, on street parking, etc. Despite, being the latest state transportation document, the highway capacity manual (HCM 2010) **[1]**, also stops addressing weaving in urban areas and provides a methodology limited to freeway c Practitioners use the HCM 2010 methodology to design and analyze alternative weaving However, using it to address urban sections is controversial.

To date, there has not been a recognized procedure for the analysis of urban weaving However, simulation seems to be a reliable and sophisticated analysis approach that is esused nowadays to address limitation of available methodologies and evaluate comple phenomenon. Hence, a simulation approach using VISSIM microscopic simulator was adopt research to address urban weaving sections.

VISSIM is a sophisticated time step and behavioral based simulation model developed to mo traffic, public transit and pedestrian flow. VISSIM offers flexibility in several respects. The s programmability overcomes the limitation of the graphical interface. In addition, the concer and connectors allows users to model geometries with any level of complexity.

4. The objective of this paper is to evaluate the capacity of Urban weaving sections is simulation models and test the appropriateness of using the HCM methodology to properational measures namely; capacity, lane change rates speed. The paper also presents ar develop several capacity predictions models and calibrate the Speed prediction algorithms 2010 using the simulation data.



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Figure 2: Flow components and the formation of weaving sections

6 In this study, a network similar to Figure 1 is coded in VISSIM and used as an exp test bed to explore the effects of weaving length, minor demand, major demand and the proj minor through on the operations of weaving section formed at intersection (1) shown in Figu analyse of both intersections and the interactions between all four weaving sections are left study. Figure 2 shows a close up of the analysed intersection and the formation of the sections-

8 At weaving section (Λ) , the minor entrance represents the on ramp and the U-turns the off-ramp. At weaving section (B), the U-turns becomes the on-Ramp and the minordownstream is the off ramp. Weaving length is measures as shown in figure 2 for both section 9

From the HCM 2010 perspective, these local weaving sections are similar characteristics to the two-sided weaving sections. The HCM 2010 methodology states that i two sided weaving sections ramp to ramp movement is the only weaving movement while movements are considered to be non-weaving. To maintain consistency with HCM, this resea adopts the same definitions of weaving and non-weaving flows

10. for both of the analysed weaving sections. 11

12. BACKGROUND

Freeway weaving has been a subject of extensive research that -aimed at improving the HCM analysis methodology. Efforts to design and analysis analyze weaving sections trace back to t fifties with the first edition of the highway capacity manual HCM 1950-HCM 1950, # contained the first weaving analysis methodology [2]. Later in 1965, the manual was update methodology was enhanced by Jack Leisch [3], introducing who introduced the concept of realm of weaving and quality of flow, which was later mapped into levels of services., which mapped into levels of service.

Over the period from 1965 to 1985, new methodologies and approaches emerged as re adopted concepts such as, the proportional use of lanes by weaving and non-weaving andvehicles, and the introduction of geometric configurations. The1985 HCM [4] incorpora new concepts and defined three types of geometric configurationsconfigurations; type A, ty Type C. Further updates were carried out in 1994 and 1997 where coefficients of the speed 1 equations were revised, and the LOS was altered to be dependent on density rather than sp HCM 2000 edition-[5] contained further improvements and introduced a multi-page tabl estimating the capacity estimation of weaving sections, which was a major improve methodology.

In 2006, the NCHRP sponsored project 3-75, which lead to the development of a new analysis methodology [6] that was later incorporated in the HCM 2010 [1]. The study util modern data using aerial photography and divided weaving sections into one-sided and t weaving sections. The new approach relied on the lane changing activity within the weaving reflect the impact of configuration and type of operation on the performance of the section, originally presented by Fazio in 1986 [7]. The methodology also provided a forwardstraightforward equation forequation to the predict, the capacity prediction of the section capacity.

AlthoughSection.

Although, various methodologies have been established to analyze weaving sections, weav are common design elements in the urban roadway system, all previous methodologies hav

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on freeway sections where vehicles operate under uninterrupted flow conditions and access to ramp locations only. Interrupted flow conditions on the other hand are more complex to However, traffic simulation is widely used as an alternative analysis tool to efficiently complex traffic operations of real world physical systems using a computer program, modeling has proven to be a reliable alternative analysis tool that allows researchers to exploi traffic phenomenon and conduct experimental analysis with real life or synthetic data computer program.

WEAVSIM is a microscopic freeway simulation program designed specifically for s weaving sections [8]. WEAVSIM was used to investigate the effect of different arrival spee overall speed and delay of ramp weaving sections and develop a regression model to predict measures of performance using a total of 243243 experimental simulation runs[8].

INTRAS is a stochastic vehicle specific time stepping simulation model devel Wicks and Lieberman **[9]**. Skabardonis, et al (1989) utilized INTRAS to simula weavingfreeway-weaving sections in California with various types of geometric configurat researchers were able to predict weaving and non-weaving speeds that closely matched field Fazio et al. (1990) also used INTRAS to simulate weaving sections and proposed the use c conflicts as a measure of effectiveness for weaving sections modelling-instead of speed **[11]**.

FRESIM is a simulation model enhanced and reprogrammed from its predecessor, INT includes enhancements to it's geometric and operational capabilities [12]. For urban α another simulator called NETSIM was design to be more compatible with the characteristics areas [13]. Nowlin (1998) used NETSIM to analyze urban two-sided weaving sections on frontage roads. He used data from multiple simulations to develop a density prediction m⁴ managed to set recommendations on the minimum and desired weaving length [14].

INTEGERATION is a microscopic model developed by Michael Van Aerade [15]. Stev (1996) estimated Capacities of the weaving sections using INTEGERATION and stated that length affects capacity only for shorter weaving sections, while the number of lanes is the mc factor affecting capacity [16]. Zhang & Rakha (2010) Used INTEGERATION to perform ε analysis of three weaving sections in Toronto, Canada and managed to obtain simulated estimates that closely matched field capacities. The study also found that weaving rat influenced the capacity even though it is not accounted forneglected in the HCM2000 model |

VISSIM is a microscopic time step and behavioral based simulation model developed to urban traffic andtraffic, -public transport operations, and flows of pedestrians [18]. The r developed at the University of Karlsruhe in Germany based on the work of R. Wiedemann [1 al (2007) used VISSIM to simulate a type B, six lane weaving section in Emeryville, Califor extremely detailed data provided by the next generation simulation team (NGSIM). The re also and used output data to generate speed-flow relationships and estimate capacities acc The study further investigated speed-flow relationships at different volume ratios and fc capacities decrease noticeably with the increase in volume ratio [20].

Another study by Fitzpatrick (2011) utilized VISSIM to investigate relationships between length, speed, and overall vehicle operations for successive ramps on Texas freeways [21] VISSIM model was calibrated and used as an experimental test bed for a total of 360 = Factors that were used to design the experiment were traffic volume, weaving length, post and proportion of volumes. Evaluation of the simulation data revealed that weaving length is-significant variable in predicting speed when included as a continuous variable that assume relationship between speed and weaving length. The study provided guidance on recordistances between ramps and used simulation and field data to develop a speed prediction ec function of geometry and traffic.

Liu et al (2012) used VISSIM to model the impact of cross weave maneuvers on the s capacity of freeways with managed Lanes [22]. The cross weave weaving maneuver is is -

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the two-sided weaving maneuver described in the HCM 2010. A test bed VISSIM model wa carry out multiple runs with different levels of mainstream demand, cross-weave demand, m number of lanes, and minimum lane changing distance (LCW-min). Speed-flow curves were using Van Aerde's Curve curve fitting and capacities were estimated for each scenarioacc. The researchers also developed a simulation based regression model to predict the reducti weaving section capacity as a function of cross weave flow rate, number of mainstream 1 weaving length with R² of 0.9837 indicating a perfect fit.

It was concluded that the simulation approach is very reliable and allows researchers to various traffic phenomenon and conduct experimental analysis with real life or synthetic data Based on the comparisons and finding of previous researches. VISSIM traffic simulator for appeared to be the most suitable software for this research due to its modeling flexit capabilities in conducting weaving related studies of freeway and urban sections.

4

15. METHODOLOGY The methodology adopted herein, is an experimental analysis with synthetic demands using microscopic <u>simulation</u> models to simulate urban weaving sections, resulting from

microscopic <u>simulation</u> models to simulate urban weaving sections, resulting from <u>intersection design</u> under various combinations of weaving lengths and volume_demande. The simulations were conducted using the default parameters of the urban driver behavior in `

Experimental analysis often involves the investigation of numerous scenarios. However, doesn't provide a scenario manager where one could predefine all the scenarios and alle model program to run them in sequence; therefore, an external programme called **AUTO**I was developed in visual Visual basic programming language to automate the simula through the COM interface feature provided by VISSIM.

The experimental designs allowed the simulation of the weaving sections at different weavir and <u>under</u>-various levels of demand <u>inputs</u>-input and volume ratios. Some of these demicombinations caused the weaving sections to reach <u>capacity</u> which capacity, which was pc out. After capacities were derived, using the simulation, -SPSS statistical package <u>was is t</u> derive develop sSimulation based <u>logarithmic and linear</u> capacity prediction models for the urban weaving sections.

Finally, tTo test the reliability of the HCM 2010 weaving analysis methodology was methodology applied to the each weaving section to test the methodology's reliability in repursant weaving operations, the methodology was applied to the local weaving sections keyKey weaving operational measures were predicted using the HCM methodology namely; and change rates, weaving and non-weaving speeds. These predicted measures were were to those of the Simulation simulation models using statistical and graphical methods. Finally, was undertaken was made to calibrate and modify the investigate the HCM 2010 speed 1 models in representing urban weaving operations when calibrated using non-linear regression using the simulation data points to reflect urban conditions.

3.1 sSimulated network

In this study, a network similar to Figure 1 was coded in VISSIM to serve as an experimenta to explore the effects of weaving length, minor demand, major demand and the percentage through split on the operations of weaving section (A) and (B) as shown in Figure 2.

The simulated network is similar to the network illustrated in Figure 1. It Consists of two co at grade intersections, 1 km apart. The simulated network depicts a typical intersection desi

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A MICROSIMULATION APPROACH TO EVALUATE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONV INTERSECTIONS IN CAIRO, Solimon, A.K. and Abo, Hashama, M.A.S. Solimon, A.K. and Abo, Hashama, M.A.S.

Soliman, A.K. and Abo-Hashema, M.A.Soliman, A.K. and Abo-Hashema, M.A.

widely adopted at major intersections <u>corridors</u> in the city of Cairo. There is no traffic control the conflict points; however, yield control is applied at the U-turns and at the major interthrough a combination of conflict areas and priority rules <u>features</u> provided by VISSIM.

The main stream has 3 lanes each lane is 3.65 meter wide, while the minor approaches hav 3.65 m each also 3.65 meter wide. U-turn slot is a single lane without any acceleration or der bays at ______ any point. The posted speed limit is 60 km/hr and a turning speed of 2(assumed _______ for all vehicles. This study focuses on the analysis of the two sections that are formed at one of the intersections only. The study also assumed similar conditions at both intersections. The simulations were conducted using the default parameter urban driver behavior in VISSIM.

15.2 experimentalExperimental design

A general factorial design was adopted. The influential factors chosen for the experiment weaving length (WL), the minor approach demand, the major approach demand, and the \underline{F} percentage of minor through split (% Mi THR).

Weaving length (WL), is-was measured as shown in Figure 2 with four levels starting meters to 400 meters with a 100-meter increment. The Next consecutive set of weaving sections are similar to weaving section (A) and (B) in and volume conditions; therefore, these sections are excluded from the research analysis,

Hypothetical traffic volumes were used in this <u>e-current</u>-study. Minor approach ranged from 100 vph to 2000 vph, with a 100 vph increment up to <u>a level of 1000</u> then the increment is increased to 200 vph (i.e. 15 volume levels). Major approach ranged from 1000 vph to 2400 vph, with a 200 vph increment (i.e. 8 volume levels ranges were chosen based on the observed animations of a few trial simulations with low volume conditions to guarantee that these volumes covered a wide range of s levels.

All minor approach volume scenarios were modeled with 20% left turn volumes. The major was modeled with a 10% major LT and $\frac{2010}{10}$ % major RT. The major approach right turns into two 10% right turns for each intersection.

Minor approach through split (%_Mi THR) waswas defined as 25% of the minor approach for the first minor through split-level split-level (% Mi THR) and then increased to 50 minor demand for the second level of (% Mi THR). i.e. (minor traffic splits = 25% T LT, 55% RT) and then changed to 50% of the minor demand for the second level i.e. (mir splits = 50% THR, 20% LT, 30% RT).

<u>ChangingIncreasing</u> the minor through <u>wassplit was</u> intended to increase thee int weaving traffic. Increasing the minor through from 25% to 50% increases the perce ramp to ramproprise ramp vehicles from 45% to 70% of the minor demand <u>approac</u>

In total, 960 combination runs were generated using a general factorial design-provided by \mathbb{N} statistical package. Each weaving length had 120 combination runs for each level of the $\underline{\%}$ split (Mi THR = 25% & and Mi THR = 50%).

Weaving section of different length and traffic splits required different configurations, configurations; therefor, two test beds corresponding to two-both lev Mi THR split for each weaving section length was-were coded. In total, 8 VISSIM models were prepared to run 120 combinations of minor and major volume levels.

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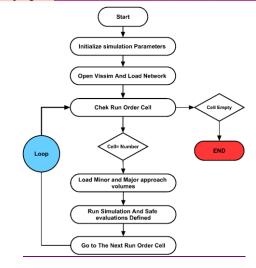
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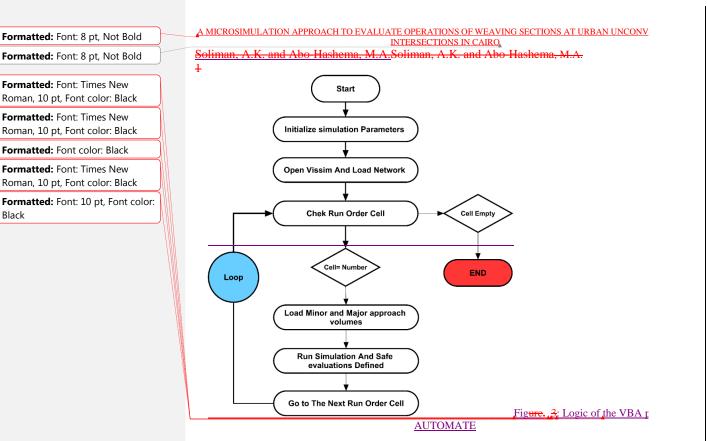
A MICROSIMULATION APPROACH TO EVALUATE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONV INTERSECTIONS IN CAIRO Soliman, A.K. and Abo Hashema, M.A.Soliman, A.K. and Abo Hashema, M.A.

15.3 <u>Automation automation program development</u>

All 960 scenarios generated by the factorial design were simulated using PTV–VISSIM $\frac{1}{2}$ traffic simulator through an external program developed in visual basic script, <u>E</u>, each needed to <u>be</u> run 3 times with different random number <u>seeds makingseeds</u>, which makes it <u>28802880</u> runs. This would have been very time consuming, therefore efficiency required at of the process using computer programming.

A computer program called **AUTOMATE**, was developed using Excel Visual Basic a automate the scenario runs generated by the factorial design. The program simply changes and minor demand volume for each <u>iterationiteration (scenario)</u> automatically using a visual next loop. Once iterations are completed, the program moves to another network and the repeated again until all networks <u>are have been</u> processed. Figure 3 depicts a flow chart that the logic of the developed program where each iteration (scenario) has a unique run order.





WEAVING SECTION MODELLING

The VISSIM network is modeled using a system of links and connectors. Conflict priority rules were used to accurately depict the interaction between crossing streams. Vehiin (vph) were loaded and changed automatically by the developed VBA program. T program immediately loads the predefined network and then the vehicle input (VISSIM is accessed through COM interface to assign vehicle inputs at the proper chos The assigned vehicle input value corresponds to a specific run (scenario) according matrix generated by factorial design and embedded within the program. Static routing to route vehicles from a start point (red) to any of the defined destinations (green) usin percentage for each destination. Routing decisions are similar for each VISSIM network. shows all routing decisions as specified in VISSIM and Table 1 summarizes the traffic splir route.

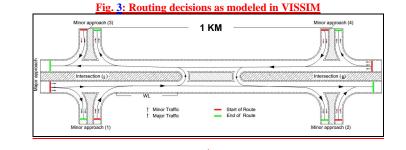


Figure 3: Logic of the AUTOMATE program for automation of simulation runs

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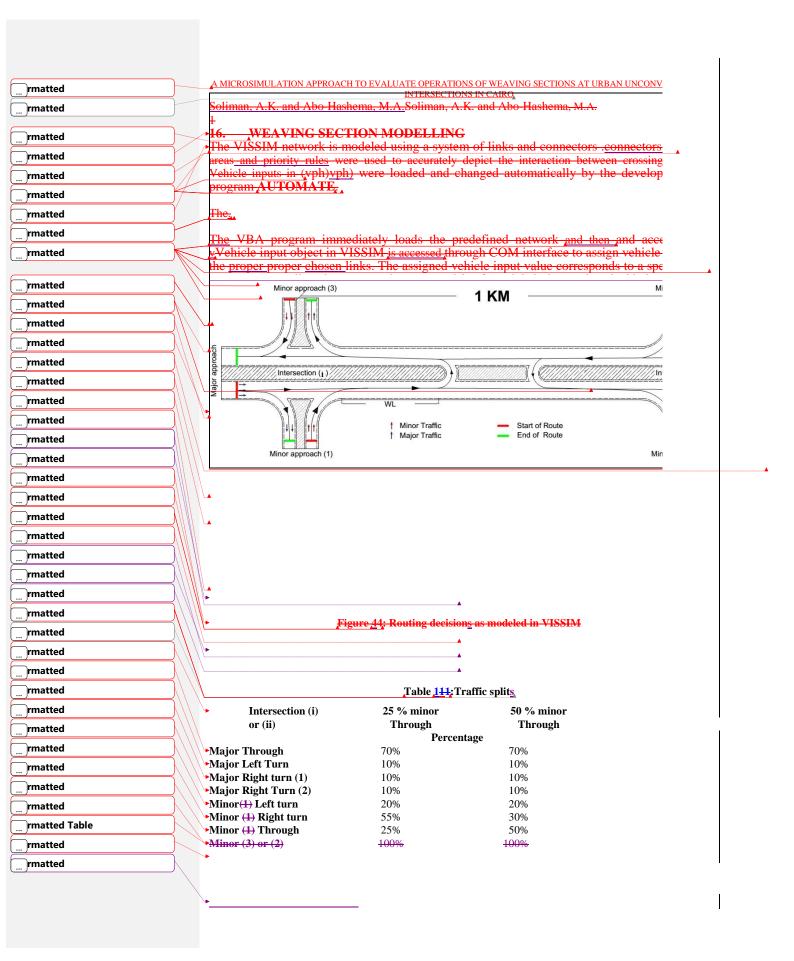
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The percentage of heavy vehicles were assumed 2% for all cases; therefore, all compositions are were 98% passenger cars and 2% <u>heavy vehicles</u>. Each vehicle type had a stochastic distribution for cars will-was be-60 km/hr (58 k km/hr) while for the heavy vehicles (HGV) the desired speed distribution will be was selec km/hr (48km/hr:58 km/hr).

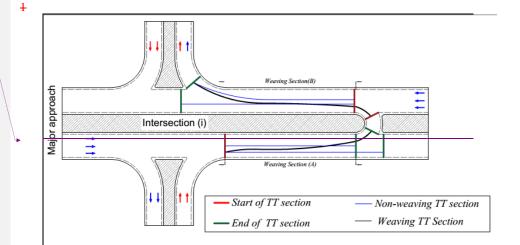
VISSIM provides a wide range of evaluations that must be defined and configured in order desired model output from the <u>a</u> simulation run. The common form of these outputs, are of files containing the results and delimited by a semicolon. The period of the each run is se seconds; however, the first 900 seconds were considered warm up and outputs were collectuat 3600 seconds of simulation. The <u>seconds</u> are offline text files containing the results.

by a semicolon. Tithe following evaluations were activated and configured in VISSIM:

- Link evaluation: The link evaluation feature <u>allows allows</u> the <u>user to gathercoll</u> simulation results based on the area of <u>the weaving for the last 3600 sec of the simula</u> as (density, Throughput, average speed).
- Lane change rates: Allows the collection of the total lane change <u>rates</u> s-executed w weaving section for the last 3600 sec of the simulation.
- Data collection Points: Collects vehicle counts and spot speeds at the major a entrance upstream of weaving section (A).
- Travel time sections: Allows the collection of travel times and vehicle <u>counts cc</u> user defined travel time section. Travel times sections are set up to capture weaving weaving <u>volumes_flow rates</u> and <u>speeds_speeds as shown</u> in figure 5as Figure_

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A MICROSIMULATION APPROACH TO EVALUATE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONV

Soliman, A.K. and Abo-Hashema.

INTERSECTIONS IN CAIRO

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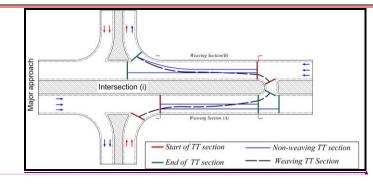


Figure Fig. 4455: Travel time sections as defined in VISSIM

From the HCM 2010 **[1]** perspective, these local weaving sections are similar in their charact the two-sided weaving sections. The HCM 2010 methodology states that in case of two sided sections the ramp-to-ramp movement is the only weaving movement, while all other move considered non-weaving. To maintain consistency with HCM, this research paper adopts definitions to differentiate between weaving and non-weaving flows. Therefore, travel time were set accordingly based on the HCM 2010 definitions of weaving and non-weaving move shown in figure 5, where the dashed line represents the weaving flow and the solid lines represents the weaving flows.

At weaving section (A), the minor entrance represents the on ramp and the U-turns represer ramp while at weaving section (B), the U-turns becomes the on-Ramp and the minor downstream is the off ramp.

Travel time sections produces the average travel time of a number of vehicles passing a use travel time section with a known distance during a specific time interval. By dividing the travel time by the length of the travel time section, the space mean speed of each movemen calculated.

17. ANALYSIS OF SIMULATION RESULTS

•After 960, simulation runs with 3 different random number seeds were conducted, a total evaluation <u>output</u> files were generated for each individual evaluation type previously conf

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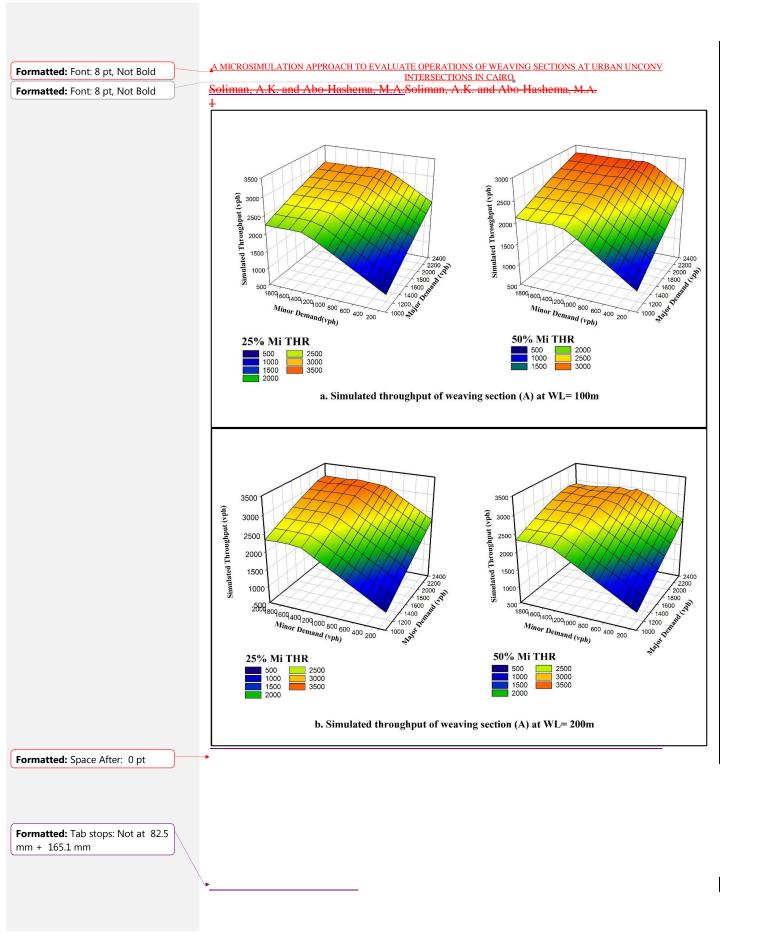
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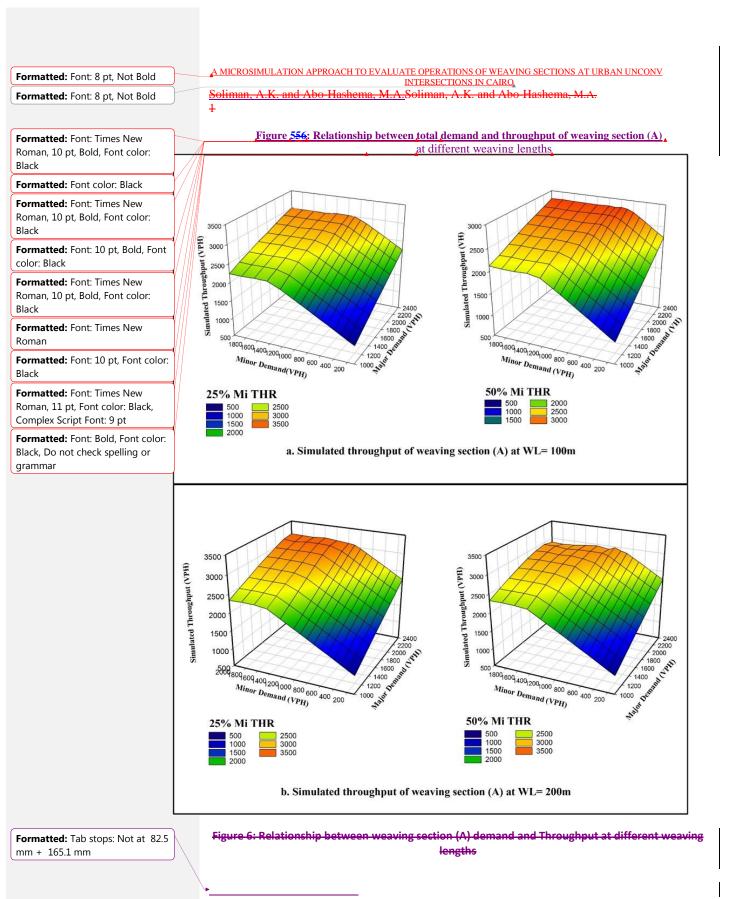
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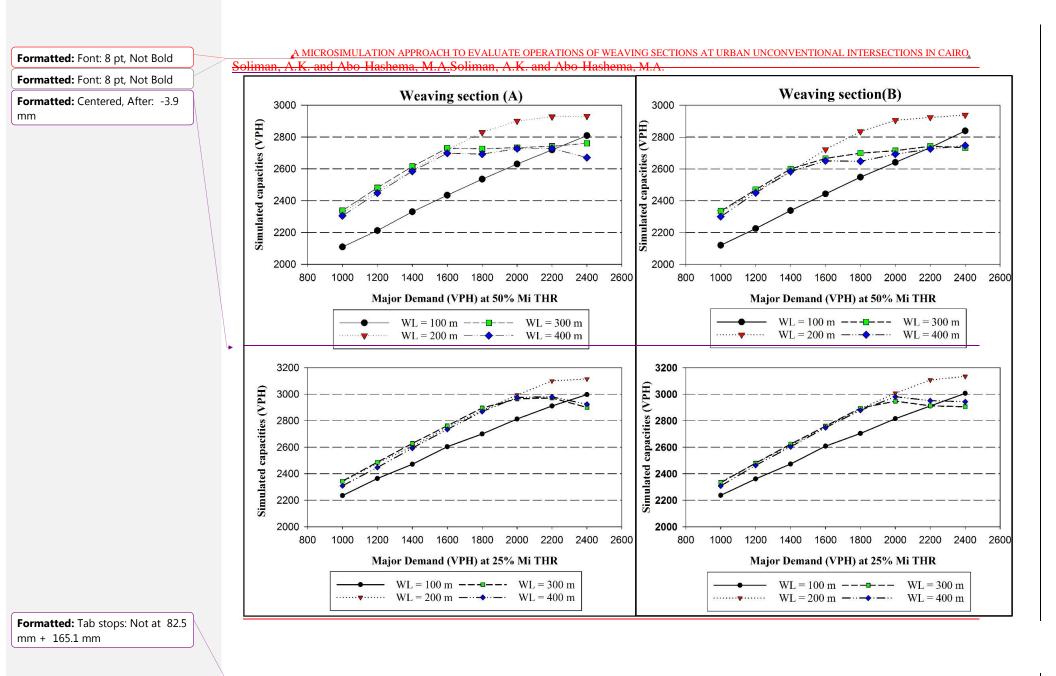
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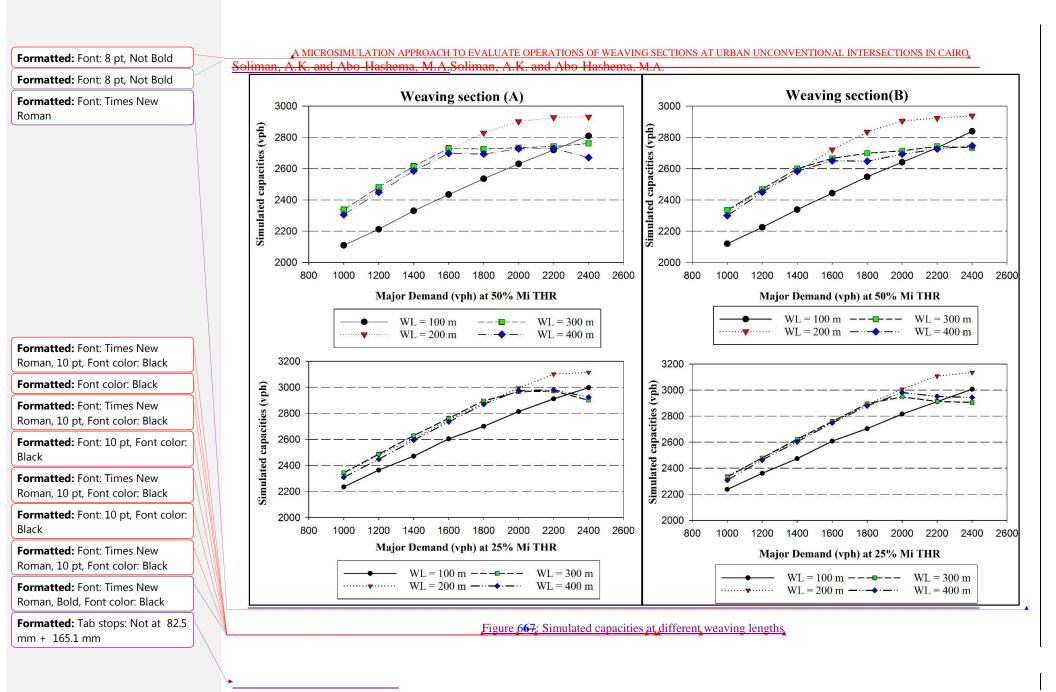
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	VISSIM (link evaluation, lane change rates, travel times, and data collection points). To l extract the simulation outputs from each file, 4 visual Visual basic Basic programs were deve the extraction and manipulation of the output data.
	<u>All-The 4</u> programs were developed using the same data extraction $logic_{\frac{1}{2}}$ however, difference is how the data was organized for each evaluation type.
	-When executing the program, it automatically prompts the user for the output folder direct choosing the <u>directorydirectory</u> , the program loops through all the <u>output</u> files importing eac Microsoft excel and <u>extracts extracting</u> the evaluation data. The program also calculates the a each repeated simulation run with different number seeds and preform <u>s any</u> necessary ca such as converting the travel times to <u>into speeds when extracting</u> the travel time converting vehicles into Passenger car equivalents.
Formatted: Space Before: 0 pt, After: 0 pt, Line spacing: Exactly 12 pt	• 5.1 capacity <u>Capacity</u> estimation using simulation VISSIM or any other simulation tool does not produce capacity estimates directly. The easing estimate Capacity is to observe the link (Study segment), throughput against an increasing
	input. The throughput represents the actual vehicles processed by VISSIM, while the represents the vehicle inputs that are assigned to the simulated network.
	The <u>At first, the</u> throughput equals the input demand up to a certain point, <u>and</u> then indicating that the system is not able to accommodate any more vehicles and is operating at <u>The throughput represents the actual processed vehicles while the demand represents the inputs that are assigned to the simulated network.</u>
	It was <u>also</u> expected to observe that the throughput of weaving section (A) is almost eq throughput of section (B). Theoretically, the throughput of weaving section (A), should be equal to the throughput of weaving section (B) assuming similar volume inputs and routing at the next consecutive intersection of the network, in addition to similar number of seeds (in simulated vehicular arrival times).
	-Figures 6 shows the relationship between the <u>total</u> demand and throughput of weaving section a <u>400100 meters</u> and <u>and 200 m200 meters</u> weaving lengths as an example. From <u>the surface</u> Figure 6, it is noticeable that the throughput increases with the increase in total demand <u>majorMajor</u>) until reaching a certain threshold where it becomes nearly constant and unresp <u>further increase in the continuous dd</u> emand <u>increase</u> . The point of maximum constant the represents the capacity of the weaving section. The figure also shows that the capacity of the directly proportional to the major demand level and to the weaving length.
	From the same figure figure, it is also noticed that the Throughput throughput decreases with increase in minor through traffic split increased from 25% to 50%. The decrease in capatitributed to the fact that increasing the percentage of minor vehicles through increased the flow from 45% of the minor demand to 70% leading to a higher VRvolume ratio (VR), which causes higher lane changing related turbulence and turbulence, and lower capacityies.
	Figure 7 shows the simulated capacities of all weaving lengths at each major demand levelength was increased beyond 200 meters, the capacities increased with <u>major</u> demand up to point and then started to level after major demand exceeded 2000 and 1600 <u>VPH-vph</u> for 50% Mi THR split, respectively for both weaving sections.
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	Let us found that increasing the weaving length beyond 200 meters did not increase the ca the sections, on the contrary capacities was less than the values associated with the shorter length (200 meter-) for major demand levels greater than 2000 and 1600 VPH-vph for at 50% Mi THR respectively for both sections.
Formatted: Space After: 0 pt, Line spacing: Exactly 12 pt	•Capacity difference between sections (A) and (B) for at both cases levels of $\frac{M}{M}$ Mi THRTHR 50%) was neglected neglected, as the maximum difference was did not exceed 2.5% maximum difference
Formatted: Font: 11 pt, Font color: Auto, Complex Script Font: 11 pt	slight variation in throughputs between Weaving section (A) and (B) is <u>only</u> attribute stochastic behaviour of the simulation model.
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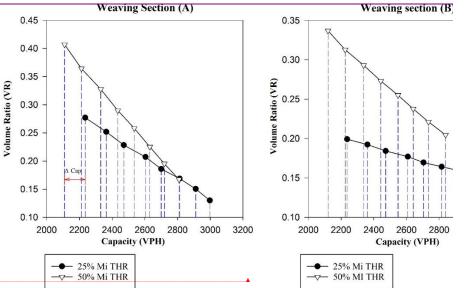
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A MICROSIMULATION APPROACH TO EVALUATE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONVI INTERSECTIONS IN CAIRO Soliman, A.K. and Abo-Hashema, M.A.Soliman, A.K. and Abo-Hashema, M.A.

The impact of volume ratio on capacity is summarized in figure 8 and 9. The points on ea represent the capacity at a certain major demand level d in Figure 8. The fFigure 8 sl relationship between capacity and volume ratio for a weaving length of 100 meters as an



Figure, 88: capacity Capacity versus volume ratio for WL= 100

The reduction in capacity for each pair of capacity estimates at different % Mi THR represented by a Δ Cap on the graph. Although capacities of section (A) and (B) are equal ratio at capacity of weaving section (B) is noticeably lower than its corresponding value of se for both levels of minor through traffic. This explains why sections (B) at some points will c a relatively higher LOS than section (A) which is why it should be treated as a separate configuration.

When the weaving length was increased beyond 200 meters as shown in figure 9, capacitie increase after major demand exceeded 2000 (vph) and 1600 (vph) for 25% and 50% respectively even though volume ratio seems to be decreasing. Form figure 9 it is also notice Δ Cap associated with a major demand < 1400 (vph) for weaving sections (A) and (B) resperiminum. Which indicates that the turbulence resulting from increasing the % Mi THR s 25% to 50% is sustained for major demand levels \leq 1400 (vph).

It is worth mentioning that when length was increased to 200 meters the simulated volui increased. However, the estimated capacities were higher than the shorter weaving sect meters) as the extra length compensates for the capacity losses due to the increase in as volu The volume ratio increases with length due to an increase in the minor entrance throughput, of weaving section (A).

When investigating the relationship between the throughput of the minor entrance, and the demand, it was noticed that capacities of the minor entrance have a negative correlation capacities of the weaving sections. The Capacities of the Minor entrance is inversely proportioned to the weaving length up to 200 meters. Increase the capacity of the minor approach a in the Figure 10.

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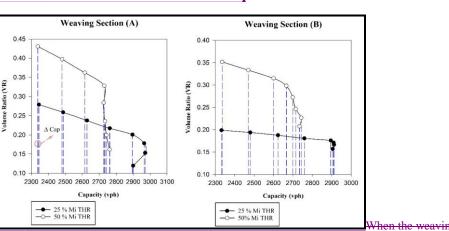
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A MICROSIMULATION APPROACH TO EVALUATE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONVI INTERSECTIONS IN CAIRO Soliman, A.K. and Abo Hashema, M.A. Soliman, A.K. and Abo Hashema, M.A.

Capacity of the weaving sections increases with the decrease in volume ratio for both cases through split. The reduction in capacity for each pair of capacity estimates at different % Miis represented by a Δ Cap on the graph. Although capacities of section (A) and (B) are equal, ratio at capacity of weaving section (B) is noticeably lower than its corresponding value of se for both levels of minor through traffic. This explains why sections (B) at some points will of a relatively higher LOS than section (A).



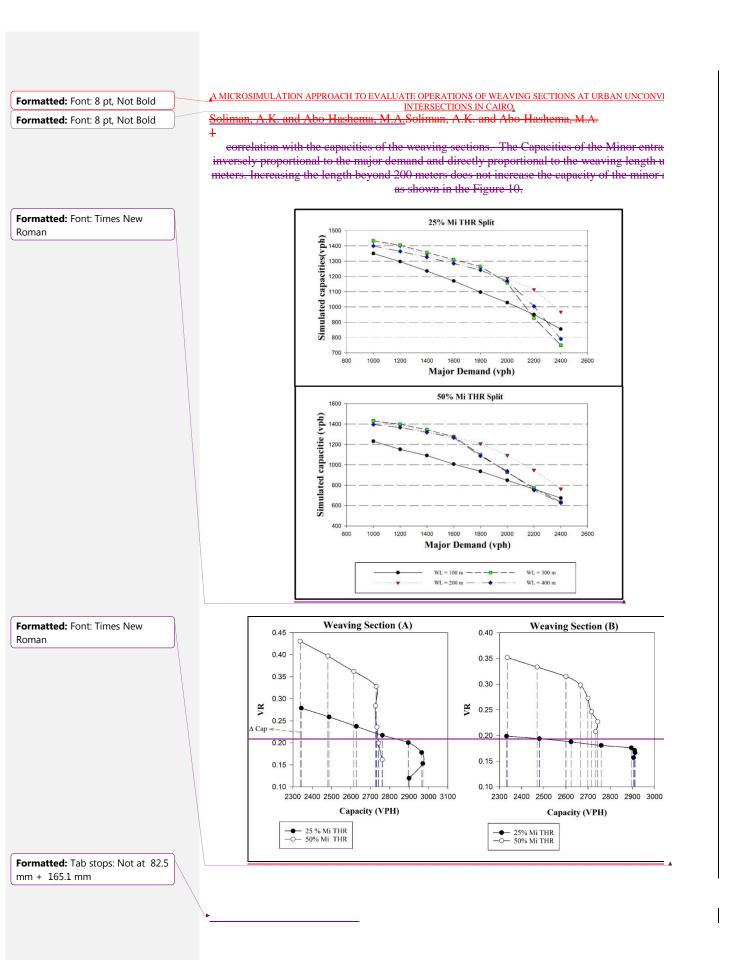
was increased beyond 200 m, capacities did not increase after major demand exceeded 2000 and 1600 (VPH) for 25% and 50% Mi THR respectively even though volume ratio seems decreasing as shown in figure 9.

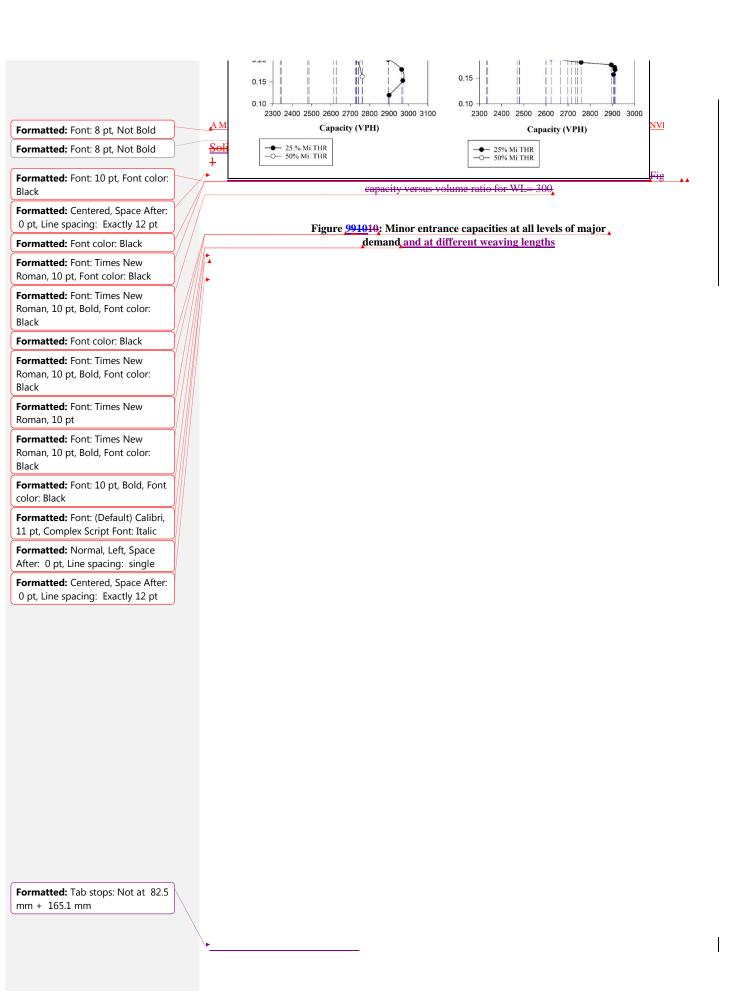
Figure 889: eCapacity versus volume ratio for WL= 300

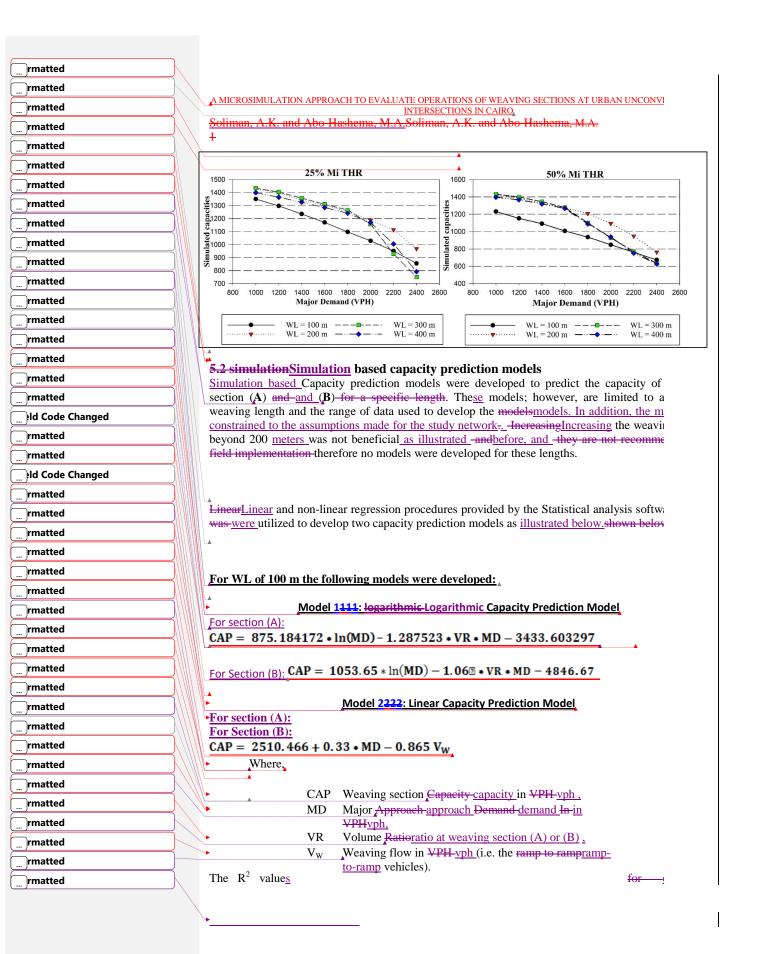
Form the same figure it is also noticeable that A Cap is associated with a major demand (VPH) for weaving sections (A) and (B) respectively is minimum. Which indicates that the tu resulting from increasing the Mi THR split from 25% to 50% is sustained for major demand 1400 VPH. After major demand exceeds these values a decrease in capacities is noticed how decrease is not more than 8% for both weaving sections.

It is worth mentioning that when length was increased to 200 m the simulated volume ratios i however, the estimated capacities were higher than the shorter weaving section (100 m) as the length compensates for capacity losses due to the increase in weaving intensity as volume increases.

The volume ratio increases with length as a consequence of an increase in minor throughput i of weaving section (A). When investigating the relationship between the throughput of the entrance and minor demand it was noticed that capacities of the minor entrance have a neg







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A MICROSIMULATION APPROACH TO EVALUATE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONVI INTERSECTIONS IN CAIRO Soliman, A.K. and Abo Hashema, M.A.Soliman, A.K. and Abo Hashema, M.A.

logarithmic models is are relatively high (0.992) which indicates a perfect fit. The negative the coefficient of volume ratio is also logical which means that when volume ratio is increased capacity decreases. The R² value of the linear models are <u>-2 is</u> also high, (0.975) and respectively. and aAll coefficient signs are also relevant indicating logical correlation bet dependent variable and the predictors.

For WL of 200 m the following models were developed:

Model 4444: Linear capacity prediction model

For section (A): CAP = 1977.41 + 0.495 * MD - 0.1V_WCAP = 1912.324 + 0.556 * MD - 0.129 V_W For section (B): CAP = 1968.56 + 0.52 * MD - 0.16 V_WCAP = 1912.324 + 0.556 * MD - 0.129 V_W

The R^2 values for of the logarithmic models is equal (=-0.983961) and (0.968) which is a high while the linear model has a R_1^2 respectively, which is relatively high, while the linear have an R_1^2 , values of of (0.969929) and (0.932) which respectively. All models coefficien relevant signs.

• is also high. All models have relevant signs indicating logical correlation between the depend variable and the predictors.

5.3 applicability<u>APPLICABILITY</u> OF <u>hem-HCM</u> 2010 WEAVING ANALYSIS METHODOLOGY

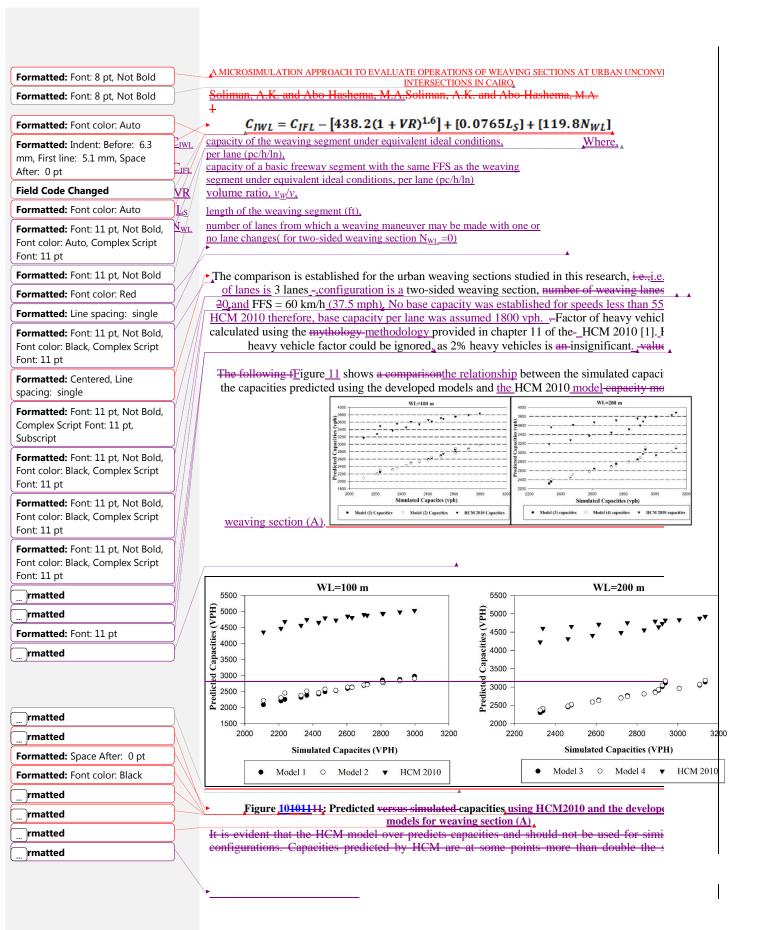
To facilitate the application of HCM, an excel based calculation sheet is <u>was</u> developed to the operational measures of weaving section (**A**) and (**B**) using the simulated weaving weaving volumes obtained from 960 simulation runs. All the algorithms embedded in the ca sheet is <u>explained found</u> in the HCM 2010 chapter 12 **[1**].

▶ **5.3.1 capacity** Capacity

•The HCM 2010 presents a straight forwardstraightforward equation to estimate the lane ca the weaving section whichsection, which in this case will be a two_sided weaving section make comparisons possible some adjustments were made to the capacity values derived 1 HCM 2010 to be more compatible with the capacities estimated using VISSIM. These adjust as following:

- Simulated capacities are provided for the entire section therefore, HCM capac multiplied by the number of lanes N= 3.
- Simulated capacities are <u>were given obtained</u> in <u>VPH vph</u> therefor; HCM c should be adjusted to <u>VPH vph</u> using the heaving vehicle factor FHV.
- Weaving Length should be in units of feat.

The following equation is used to estimate the capacity of the weaving section in HCM 2010:



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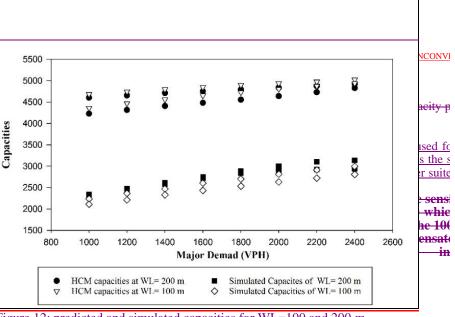


Figure 12: predicted and simulated capacities for WL=100 and 200 m

Attempts to calibrate the HCM model structure to fit the simulated capacities using non regression yielded very low \mathbf{R}_{λ}^2 values indicating that the model structure itself is not suit this type of urban weaving section and the develop model structure clearly is better and suitable for this type of weaving section.

5.3.2 laneLane change rates

Where,

Th

The speed prediction models of the HCM 2010 mainly depends on the rate of lane changes v weaving section of study. The total lane-changing rate of all vehicles in the weaving s computed by combining weaving lane change rates and non-weaving lane change.

The model for predicting weaving lane changing rate in HCM 2010 is as following:

$\underline{LC_{MIN}} = \underline{LC_{RR}} \times \underline{V_{W}}$

<u>LC_W= LC_{MIN} +0.39 [(L_S-300)^{0.5} N² (1+ ID)^{0.8}]</u>

<u>LC_{MIN}</u>	Minimum equivalent hourly rate weaving vehicles must make to successfully
<u>LC_{RR}</u>	<u>complete all weaving maneuvers</u> Minimum number of lane changes that must be made by one ramp to ramp
<u>DCRR</u>	vehicle to execute the desired maneuver successfully
$\underline{V}_{\underline{W}}$	Weaving volume in pc/h
<u>LC_W</u>	Equivalent hourly rate at which weaving vehicles make lane changes within the weaving section (Lc/h);
L <u>s</u> N ID	Length of the weaving section (ft); Number of lanes within the weaving section; Interchange density.
	the urban two-sided weaving sections studied in this research $LC_{RR} = 2$, and ID is
<u>0.67</u>	<u>.</u>
The mo	del for predicting non-weaving lane changing rate in HCM 2010 is as following:
First	, a non-weaving vehicle index, "I _{NW} , needs to be calculated and then depending of 1
	non-weaving lane changing rates are calculated using one of the three non-weav
chan	ge rate equations below.

rmatted rmatted A MICROSIMULATION APPROACH TO EVALUATE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONVI rmatted INTERSECTIONS IN CAIRO and Abo-Hashema, M. Soliman, A.K. and Abo -Hashema, M.A. $\frac{\text{Soliman, A.K. and Ab}}{I_{NW}} = \frac{L_{S} * ID * V_{NW}}{10000}$ rmatted rmatted 10000 rmatted <u>If I_{NW}≤1300</u> then $\underline{LC}_{NW} = \underline{LC}_{NW1}$ rmatted **If I**_{NW}≥ 1950 $LC_{NW} = LC_{NW2}$ then **If** $1300 < I_{NW} < 1950$ then $\underline{LC}_{NW} = \underline{LC}_{NW3}$ rmatted rmatted $LC_{NWL} = (0.206*V_{NW}) + (0.542*L_{S}) - (192.6*N)$ $LC_{NW2} = 2,135 + 0.223(V_{NW} - 2,000)$ rmatted $LC_{NW3} = LC_{NW1} + (LC_{NW2} - LC_{NW1}) * ((1/650) (I_{NW} - 1300))$ rmatted The total lane-changing rate of all vehicles in the weaving section is then computed by co rmatted LC_w and LC_{NW.} LC_{ALL}=LC_W+LC_{NW} rmatted rmatted In VISSIM, lane change evaluations were set to calculate the total number of lane changes rmatted that occurs at each weaving section. Paired t-tests were executed using SPSS statistical packa rmatted the null hypothesis which hypothesis, which states that there is no significant difference bet simulated and the predicted lane changes at section (A) and (B) for each weaving length. rmatted rmatted Parity plots of the simulated versus predicted estimates were also generated, which gives description of the relationship between the observed simulated and the predicted values. T rmatted the plot of points to the 45° Line, the more accurate the predicted speeds. If points are on the rmatted the 45° Line, predicted lane changes are generally larger while predicted lane changes are 1 values on the right of the line. rmatted Parity plots are shown in figure 12 for weaving length of 100 and 200-asmeters as an ex rmatted Figure 13. By examining the plots, it is evident that there is a moderate to high magnitude (degree of a high positive correlation between the predicted and the simulated values. Clearly rmatted the points are to the right of_-the 45°-degree line whichline, which indicates that the simul rmatted change rate is higher than the predicted. The same analogy can be applied on the rest of the rmatted lengths in this study. The following table shows the result of paired t-test with a significance value < 0.05 indicates the state of the state rmatted there is a statistically significant difference between the simulated and the predicted valuessi rmatted the predicted values, and thereby rejecting the null hypothesis. rmatted rmatted rmatted Paired Differences rmatted Mean rmatted Confidence Samples Section Error Std. ML Interval of the rmatted Lane change rates Mean Deviation Difference rmatted Std. Lower rmatted rmatted Simulated_LC -Pair 1 Α 63.64 257.64 16.63 30.87 HCM _LC 8 rmatted Simulated_LC rmatted Pair 2 В 130.71 137.49 8.87 113.22 HCM _LC rmatted Simulated_LC -Pair 3 396.49 317.66 20.50 356.09 Α HCM _LC rmatted 200 Simulated_LC -

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Pair 5

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A MICROSIMULATION APPROACH TO EVALUATE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONVI INTERSECTIONS IN CAIRO

<u>Soliman, A.K. and Abo Hashema, M.A.</u>Soliman, A.K. and Abo Hashema, M.A. ⊥

Pair 6		В	Simulated_ LC – HCM _LC	646.39	391.31	25.26	596.63	696.15	25.59	239.00	.00
Pair 7	0	А	Simulated_LC – HCM _LC	352.22	411.38	26.55	299.91	404.53	13.26	239.00	.00
Pair 8	40	В	Simulated_ LC – HCM _LC	524.27	430.23	27.77	469.57	578.98	18.88	239.00	.00

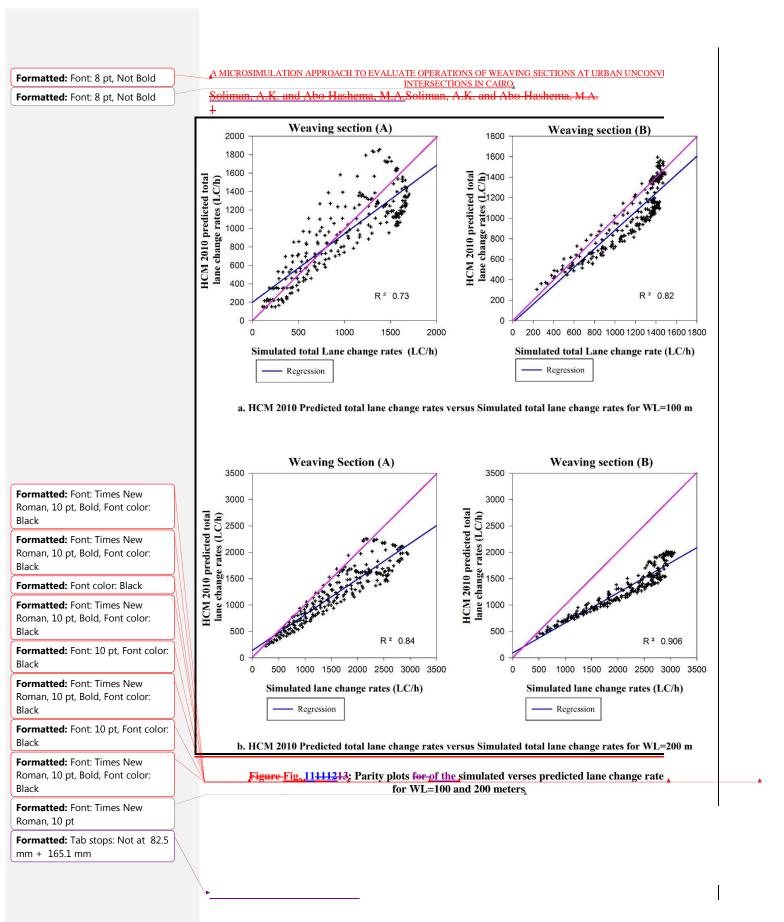
Table <u>222</u>: Lane change rates Paired Samples Test

From the previous <u>section_section</u>, it was concluded that the lane change prediction models by HCM 2010 clearly under-predicts the rate of lane changes for this type of urban weaving Consequently, the predicted speeds are likely to be higher than the actual speeds.

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A MICROSIMULATION APPROACH TO EVALUATE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONVI INTERSECTIONS IN CAIRO Soliman, A.K. and Abo-Hashema, M.A. Soliman, A.K. and Abo-Hashema, M.A.



Formatted: Font: 8 pt, Not Bold	A MICROSIMULATION APPROACH TO EVALUATE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONVI INTERSECTIONS IN CAIRO,
Formatted: Font: 8 pt, Not Bold	Soliman, A.K. and Abo Hashema, M.A. Soliman, A.K. and Abo Hashema, M.A.
Formatted: Font: (Default) Calibri, Not Bold	5.3.3 weaving Weaving and non-weaving speed (s _w , s _{nw})
Formatted: Space Before: 6 pt, Line spacing: single	•The HCM_2010 presented two models for the prediction of weaving and non-weaving speec
Formatted: Space After: 0 pt	models and dependant on the rate of lane changes where speeds would decrease with increas changing activity – a real measure of weaving intensity. It is worth mentioning that the prec
rmatted	non-weaving vehicle speed is the weakest part of the HCM 2010 methodology <u>[6]</u> .
ld Code Changed	$S_W = 15 + \frac{FFS - 15}{1 + W}$, $W = 0.226 * \left(\frac{LC_{ALL}}{L_S}\right)^{0.789}$ (Weaving speed prediction
rmatted	$\frac{J_{W} - IJ + 1}{1 + W}$, $\frac{J_{L_{S}}}{L_{S}}$ (Weaving speed prediction
Field Code Changed	$S_{NW} = FFS - (0.0072 * LC_{MIN}) - \left(0.0048 * \frac{V}{N}\right) $ (Non-weaving speed prediction to
rmatted	(Non-weaving speed prediction 1
rmatted	Where,
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rmatted	Snw Average speed of weaving vehicles in (mi/h); Snw Average speed of non-weaving vehicles in (mi/h);
rmatted	Snw Average speed of non-weaving vehicles in (mi/h); LC _{Min} Minimum lane change rate depending on geometry

- LC_{Min} Minimum lane change rate depending on geometry
- LC_{ALL} Total rate of lane changes
- Number of lanes in the weaving section Ν
- FFS Free flow speed

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- V Total flow rate in pcu
- W Weaving intensity factor
- Total flow rate in pcu V
- LS Weaving length in ft

In VISSIM, travel time sections were utilized to capture the weaving and non-weaving si both weaving section (A) and (B) and . The travel time were sections were set based on t definitions of weaving and non-weaving movements to maintain consistency when e comparing simulated and predicted samples.

All HCM predicted weaving and non-weaving sSpeeds that were estimated using HC previously converted into Km/h for consistency aswell. in the HCM 2010 calculation sheet p developed for this study.

A Paired sample t-test was carried out to compare the simulated and predicted weaving weaving speeds and parity plots were also were developed generated for each weaving Figure length. Figure 14-13 shows parity plots for weaving length of 100 and 400 mete example -.

As expected the HCM models over predicted the speeds of all movements as shown in the p Goodness of fit (R²) was very small and all the plotted points were on the left side of the 45 lineline, which means that the predicted speeds are higher.

It is also noticeable that the simulated speeds were more spread compared to that of the speeds, speeds; however, there is a significant degree of linearity and a positive correlat weaving and non- weaving speeds failed the T-test with significance value ≤ 0.05 and concluded that the results speeds were significantly different as shown in Table 3,

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Formatted: Font: Bold					<u>Weaving and non-</u> weaving speeds		Pair	ed Diff	erences		t		
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	$\left\{ \left \right\rangle \right\}$	Pair 3	M		Simulated Sw –HCM Sw	-21.41	1.48	.10	-21.60	-21.22	-223.38	239.00	Ι.
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	Ĩ\\	Pair 4			Simulated_Snw –HCM Snw	-14.68	4.34					239.00	Ι.
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A MICROSIMULATION APPROACH TO EVALUA	TE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONVI
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Soliman, A.K. and Abo-Hashema, M.A.	Soliman, A.K. and Abo-Hashema, M.A.

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Pair 11			Simulated Sw –HCM Sw	-27.11	3.75	.24	-27.59	-26.63	-112.00	239.00	.00	
Pair 12	E	В	Simulated_Snw –HCM Snw	-14.80	11.58	.75	-16.28	-13.33	-19.80	239.00	.00	
Pair 13			Simulated Sw –HCM Sw	-11.48	11.17	.72	-12.90	-10.06	-15.91	239.00	.00	
Pair 14	=400	A	Simulated_Snw –HCM Snw	-4.47	13.04	.84	-6.12	-2.81	-5.31	239.00	.00	
Pair 15	ML	5	Simulated Sw –HCM Sw	-12.79	7.24	.47	-13.71	-11.87	-27.37	239.00	.00	
- P air 16		В	Simulated_Snw –HCM Snw	-13.07	11.93	.77	-14.59	-11.56	-16.97	239.00	.00	

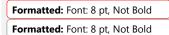
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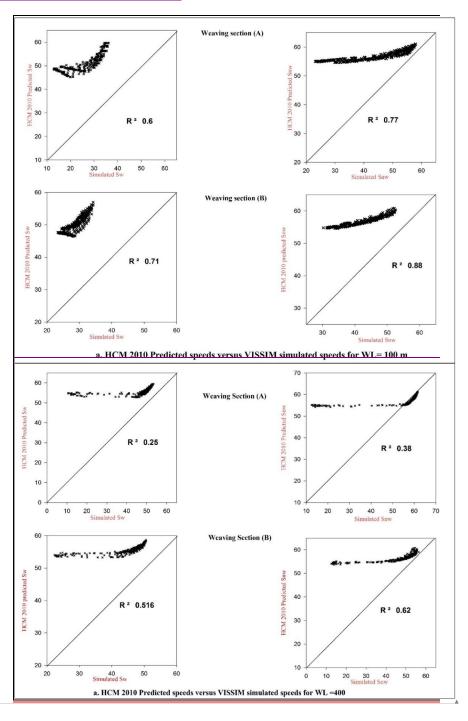
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A MICROSIMULATION APPROACH TO EVALUATE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONVI INTERSECTIONS IN CAIRO Soliman, A.K. and Abo Hashema, M.A.Soliman, A.K. and Abo Hashema, M.A.



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A MICROSIMULATION APPROACH TO EVALUATE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONVI INTERSECTIONS IN CAIRO Soliman, A.K. and Abo Hashema, M.A.Soliman, A.K. and Abo Hashema, M.A.

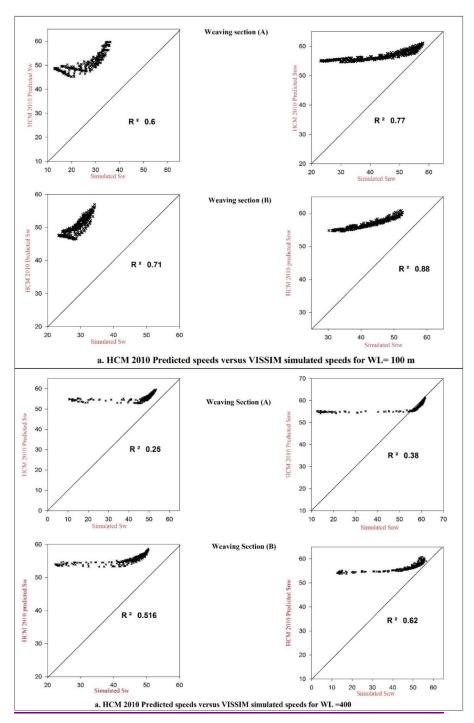


Figure <u>12121314</u>; Parity plots for <u>of simulated the simulated</u> verses <u>the</u> predicted weaving and weaving Speeds

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A MICROSIMULATION APPROACH TO EVALUATE OPERATIONS OF WEAVING SECTIONS AT URBAN UNCONVI INTERSECTIONS IN CAIRO Soliman, A.K. and Abo-Hashema

5.4-Calibrating the HCM speed prediction modelmodelss

Using the non-linear regression procedure in SPSS, an effort was undertaken to calibrate an the HCM speed prediction algorithms with using the simulation data points to represe conditions more accurately with the hope of representing better urban conditions. The model original forms use a minimum speed of upper and lower 15 mph and a maximum speed-speed to to the free flow speed FFS(FFS).

-The minimum value observed of from simulated the simulation modelsspeeds for the urban sections was were lower than 15-mi mph/hr,; therefore therefore, the original models were mo using the actual observed minimum speed of 6 mph-mi/h. Also all lengths were used in units

Each weaving section was considered as a separate configuration. Free flow speed (FFS) valu mi/h (60 km/h) was used for substituting the FSS FFS in SPSSSPSS; therefore, therefore, the format was used as following:

$$S_W = 6 + (FFS + -a_0) / (1 + (b^*(L_{CALL}/L_S)^{C}))$$

Calibrated Weaving speed prediction model for Section (A),

	$S_W = 6 + \frac{FFS - 6}{1 + W}$	w h ere	$WwW = 0.322437 * \left(\frac{LC_{ALL}}{L_{S}}\right)^{1.386716}$
--	-----------------------------------	----------------	---

The calibrated model resulted in R^2 of 0.54 which 0.54, which is not bad when compared to a 0.614 obtained by freeway based freeway-based data [6].

Calibrated Weaving speed prediction model for Section (B)

 $WwW = 0.557494 * \left(\frac{LC_{ALL}}{L_{S}}\right)$ 0.787467 $S_W = 6 + \frac{FFS - 6}{1 + W}$ where

The calibrated model resulted in an R² of 0.343-compared to the previous calibrated model.

Attempts to calibrate the non-weaving speed prediction model failed to produce any si results, which was quite expected given that, the non-weaving speed prediction model was ϵ a weak part of the ____ HCM 2010 weaving analysis methodology methodology. [6].

In view of the above, it is evident that the structure of the model gives a very low \mathbb{R}^2 even fo (A). On the other hand, efforts to calibrate the non-weaving speed prediction model provided 2010 failed completely tTherefore, it is not recommended for any future Attempts.

In view of the above, it is evident that speed prediction is extremely difficult and rarely 1 statistically acceptable models.

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28. CONCLUSION

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This paper presented an effort to model and simulate urban weaving sections resulting unconventional intersection design adopted widely in Cairo, Egypt. Due to this design type lack of traffic control, merging of minor traffic into the mainstream and its ability to crossgreatly affected by major approach demand. Higher major approach volumes produces lesser the minor merging traffic, therefore lesser minor approach throughput and capacity. Each major demand was associated with a maximum throughput that can propagate through the ramp i.e. capacity of the entrance ramp.

It was found that at some point, continuous increase in weaving length does not necessarily traffic operations and could reduce the capacity and LOS. It was also concluded that 200 met optimum length for maximum capacity and throughput of the weaving sections and the entrance. However, it is recommended to try to experiment lengths between 200 and 300 n future work. It is worth mentioning that Michigan department of transportation (MDOT) recer a weaving length of 200 ± 30 m as the optimum weaving Length for efficient operations whik with the findings of this research.

Form the comparative analysis with HCM 2010 methodology, it was concluded that, t methodology for these types of urban weaving configurations willwould produce unreliabl HCM 2010 models are more likely to predict lower lane changing rates, overestimated capac higher speeds. This is probably attributed to the fact that the prediction models of the HCM 2 designed for freeway conditions which conditions, which naturally involves lower rates of lower lane changes, higher speeds and higher capacities compared to urban conditions. It was also concluded that the HCM capacity model structure fails to compensate the capacidue to the increase in weaving ratio when length increases.

Efforts to calibrate the speed prediction algorithms revealed that the original structure of the I not suited to represent weaving operation even when fitted and calibrated with the simula points. It was also concluded that speed prediction of weaving operations is <u>extremely no</u> difficult, and rarely <u>results produces</u> in "statistically acceptable" results.

Utilizing this intersection design at areas with high crossing demand volumes are likely to ca back into the minor approach and enhances the chances of forming a bottleneck at the conf between the arterial and the crossing.

-At high crossing demands, mainstream vehicles penetrates through the section with very lo and could be forced to queue with the U-turning vehicles until sufficient gap arises. There efficient weaving operations the design should be implemented at areas with low crossing conditions and cannot be used at major intersections.

This study assumed similar volume condition at both intersection of the simulated Intersection with different volume conditions will results in a more complex weaving operation more complex weaving operation with 4 weaving section interacting with each other. This ca for future work.

